



114-126 MILITARY ROAD PORT KEMBLA PLANNING PROPOSAL TRAFFIC IMPACT ASSESSMENT

FOR

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1. INTRODUCTION

1.1 BACKGROUND

Bitzios Consulting has been engaged by Mr Olly Vujic to undertake a traffic impact assessment for the planning proposal to enable medium-density residential development on the former Port Kembla Public School site. This site is located at 114-126 Military Road (Lot 1 DP811699), Port Kembla, as shown in Figure 1.1.



Source: Google Earth

Figure 1.1: Site Location

1.2 PROPOSED DEVELOPMENT

The planning proposal is to propose that the site be rezoned from B4 Mixed Use to R3 Medium Density Residential and RE2 Private Recreation. This reduces minimum lot sizes from 1,999m² to 299m² and facilitates the proposed medium-density residential development consisting of:

- 7 small lot houses fronting Marne Street to the south of the site;
- 64 medium-density townhouse / terrace style residential dwellings through the middle of the site; and
- 36 residential flat dwellings at the northern end of the site.

The proposed small lot housing is to have vehicular access from individual driveways off Marne Street. An access will also be provided off Reservoir Street for the townhouses / terrace houses. Access to the proposed residential flats will be via Electrolytic Street. A copy of the development plans is provided in Appendix A.

1.3 SCOPE

The scope of this assessment consists of the following:

- a review of the existing road network and traffic conditions in proximity to the site;
- estimation of background traffic volumes at year of completion and 10-years post completion;
- calculation of the development's traffic generation and estimation of the distribution onto the external road network;
- undertake detailed traffic analysis using SIDRA Intersection analysis software to assess the capacity of the Military Road / Marne Street and Military Road / Church Street intersections at the year of opening and the 10-year design horizon;
- qualitatively assess the surrounding local access streets to determine the required road cross sections;
- advise on the location and form of all site accesses including sight distance checks;
- assessment of the development's parking and servicing requirements in accordance with Council's Development Control Plan;
- advise on the location and form of the bus stop on the development's Military Road frontage; and
- assessment of the public transport, pedestrian and cycling networks and connectivity within the vicinity of the site.

2. EXISTING CONDITIONS

2.1 ROAD NETWORK

Details of the road network surrounding the subject site is shown in Table 2.1.

Table 2.1: Surrounding Road Network

| Road Name | No. of Lanes | Speed Limit | Divided | Jurisdiction | Hierarchy | Comments |
|---------------------|--------------|-------------|---------|-------------------------|----------------------|---|
| Military Road | 2 | 60 | No | Wollongong City Council | Collector Street | Collector Road fronting the north of the site connecting to Five Island Road / State Route B65 |
| Marne Street | 2 | 50 | No | Wollongong City Council | Local Street (major) | Minor residential street fronting the south of the site |
| Church Street | 2 | 60 | No | Wollongong City Council | Collector Street | Collector Road connecting Military Road to Northcliffe Road, the east-west connector between Port Kembla and the M1 |
| Reservoir Street | 2 | 50 | No | Wollongong City Council | Local Street (major) | Minor residential street fronting the east of the site |
| Electrolytic Street | 2 / 1 | 50 | No | Wollongong City Council | Access Street | Minor access fronting the North of the site. Narrows to one-way east of Reservoir Street |

2.2 EXISTING BACKGROUND TRAFFIC

Background traffic volumes were obtained from intersection counts undertaken by Matrix Traffic and Transport Data for the Military Road / Marne Street and Military Road / Church Street intersections on Friday 2nd February 2018. The background traffic volumes surveyed for the AM (8:00-9:00) and PM (15:00-16:00) peaks are presented in Figure 2.1. The traffic survey data has been attached as Appendix B.

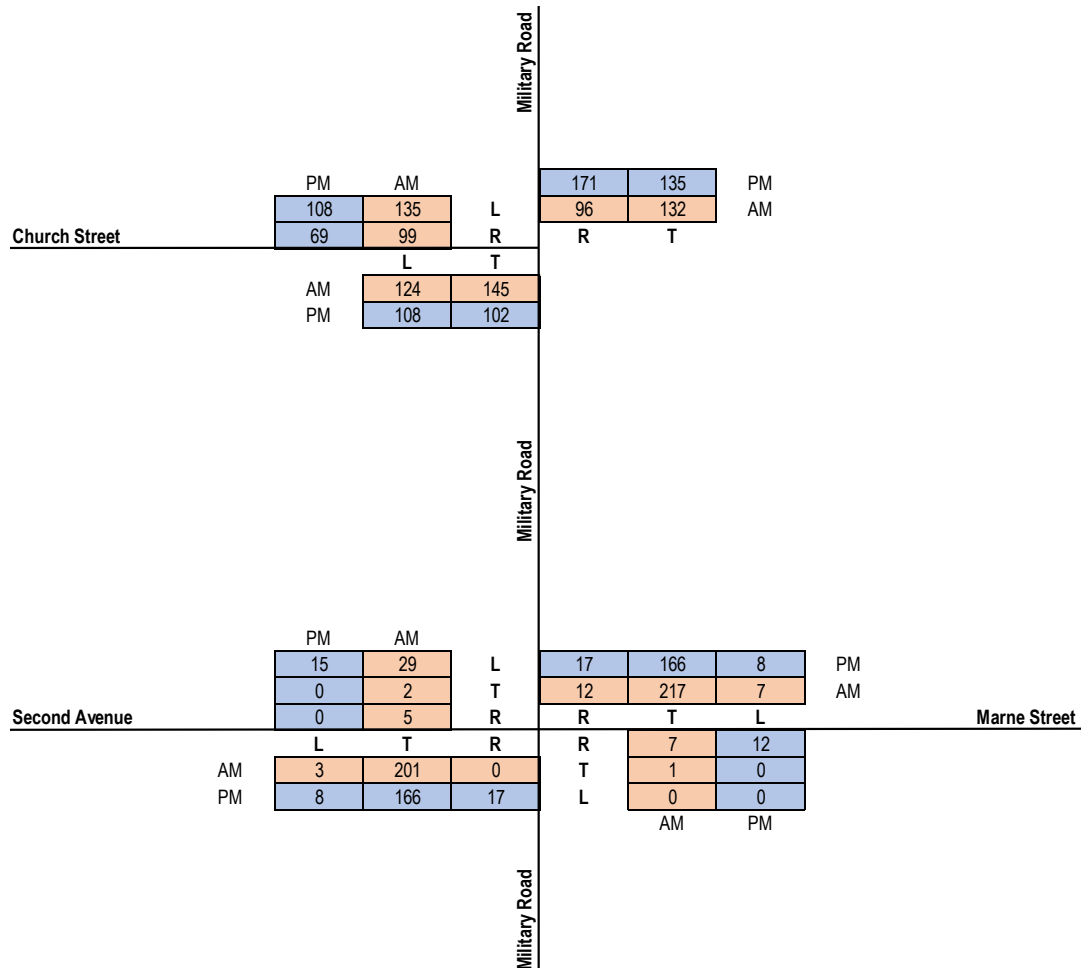


Figure 2.1: 2018 Background Traffic Volumes

2.3 PUBLIC TRANSPORT

Several bus services are provided in proximity to the subject site. Details of bus stops located within a reasonable walking distance (<400m) are provided in Table 2.2.

Table 2.2: Surrounding Bus Stop Summary

| Bus Stop ID | Location | Approximate Distance from Site | Service |
|-------------|---|--------------------------------|----------------------------------|
| 250544 | Military Road (at Marne Street) | 70m | Route 43 Route 65 |
| - | Military Road after Second Avenue | 40m | Route 43 |
| 250572 | Wentworth Street Before Church Street | 290m | Route 43 Route 65 |
| 250523 | Port Kembla Community Centre, Wentworth Road | 300m | Route 34 Route 43 Route 65 |
| 250545 | Military Road (at Quarry Street) | 350m | Route 43 Route 65 |
| 250521 | Military Road (Opposite Quarry Street) | 350m | Route 34 Route 43 Route 65 |

These bus stops identified are serviced by a number of different bus routes. Service details of these bus routes are shown in Table 2.3.

Table 2.3: Surrounding Bus Services

| Service | Route | Frequency | | | |
|---------|---------------------------------|--------------|------------------|------------|----------|
| | | Weekday Peak | Weekday Off-peak | Saturday | Sunday |
| 34 | Wollongong to Warrawong | 20 minutes | 30 minutes | 30 minutes | Hourly |
| 43 | Port Kembla to Dapto District | Hourly | Hourly | Hourly | 2 hourly |
| 65 | North Wollongong to Port Kembla | Hourly | Hourly | 2 hourly | 2 hourly |

Hourly train services are also available from Port Kembla Train Station located approximately 1.2km north of the proposed development site. This station is serviced by the South Coast Line providing access from Bomaderry / Port Kembla to Central and Bondi Junction.

2.4 ACTIVE TRANSPORT

Limited pedestrian footpaths are present within the immediate vicinity of the subject site. An existing pedestrian crossing is present on Military Road between Church Street and Third Avenue however kerb ramps are not provided for this crossing. There are also no dedicated on-street cycle lanes on roads in the local network. However, line marking indicating bicycle awareness zones are present in some areas and the additional carriageway width on Military Road may encourage cyclist usage. Existing active transport facilities are shown in Figure 2.2.



Source: Google Earth

Figure 2.2: Existing Active Transport Facilities

3. TRAFFIC ASSESSMENT

3.1 GROWTH

Considering the future expected growth of Port Kembla as outlined in the Illawarra Shoalhaven Regional Plan, and potential redevelopment or change of use of unutilised steel works buildings, some growth is expected in the vicinity of the site. Roads and Maritime Services (RMS) average annual daily traffic (AADT) counts for Illawarra Street show a linear increase of approximately 1.3% linearly in total traffic volumes from 2012 to 2017. The counter is located approximately 500m west of the Military Road / Church Street intersection as shown in Figure 3.1.



Source: Google Earth

Figure 3.1: Counter Location on Illawarra Street

As such, a linear growth rate of 1.5% p.a. has been applied uniformly to the surveyed background traffic volumes to conservatively forecast 2019 and 2029 background traffic volumes. Given the surrounding residential streets are mostly developed, this rate is considered appropriate for traffic modelling.

3.2 FORECAST BACKGROUND TRAFFIC VOLUMES

The forecast background traffic volumes for 2019 and 2029 are provided in Figure 3.2 and Figure 3.3 respectively.

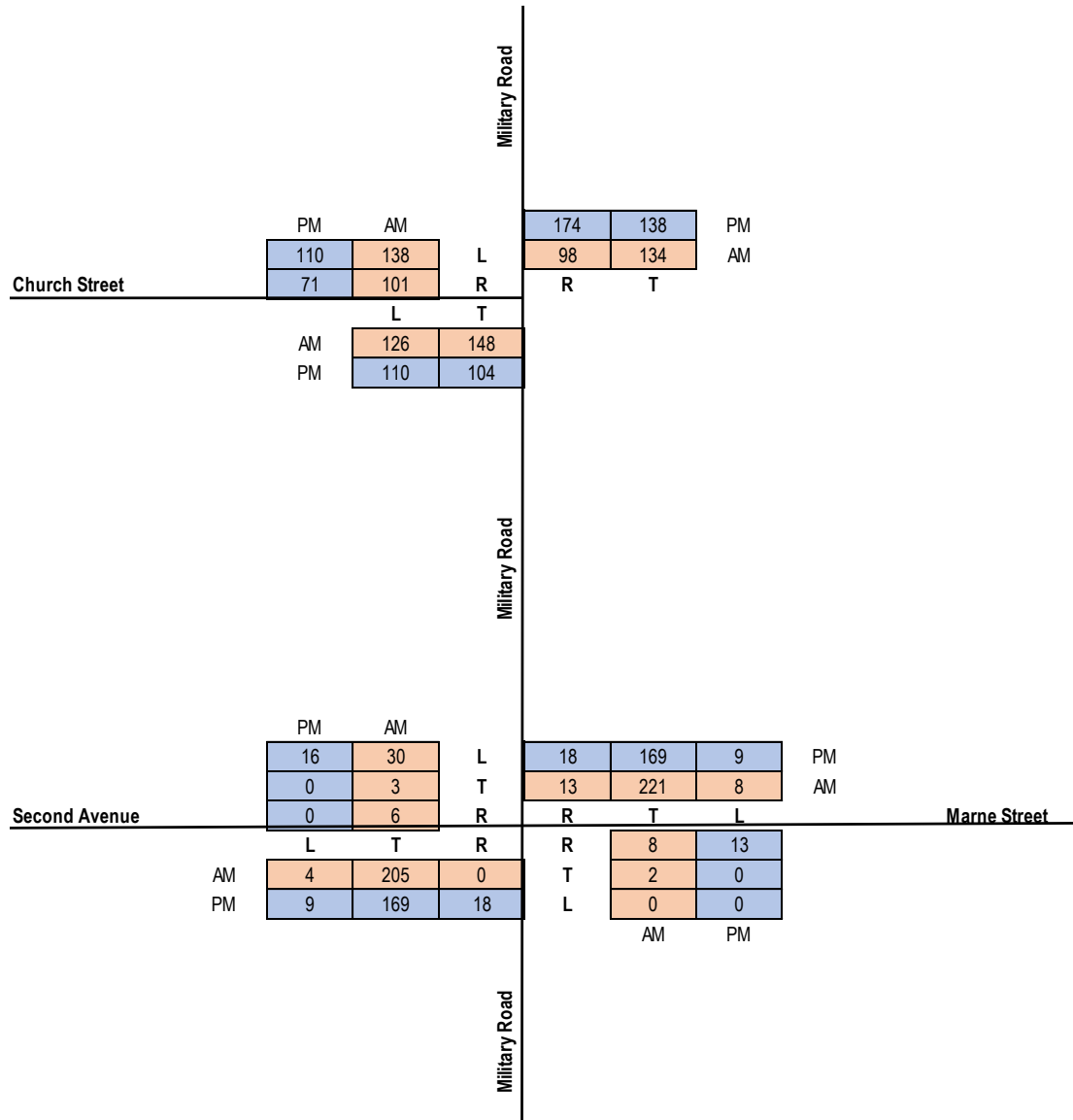


Figure 3.2: 2019 Forecast Background Traffic

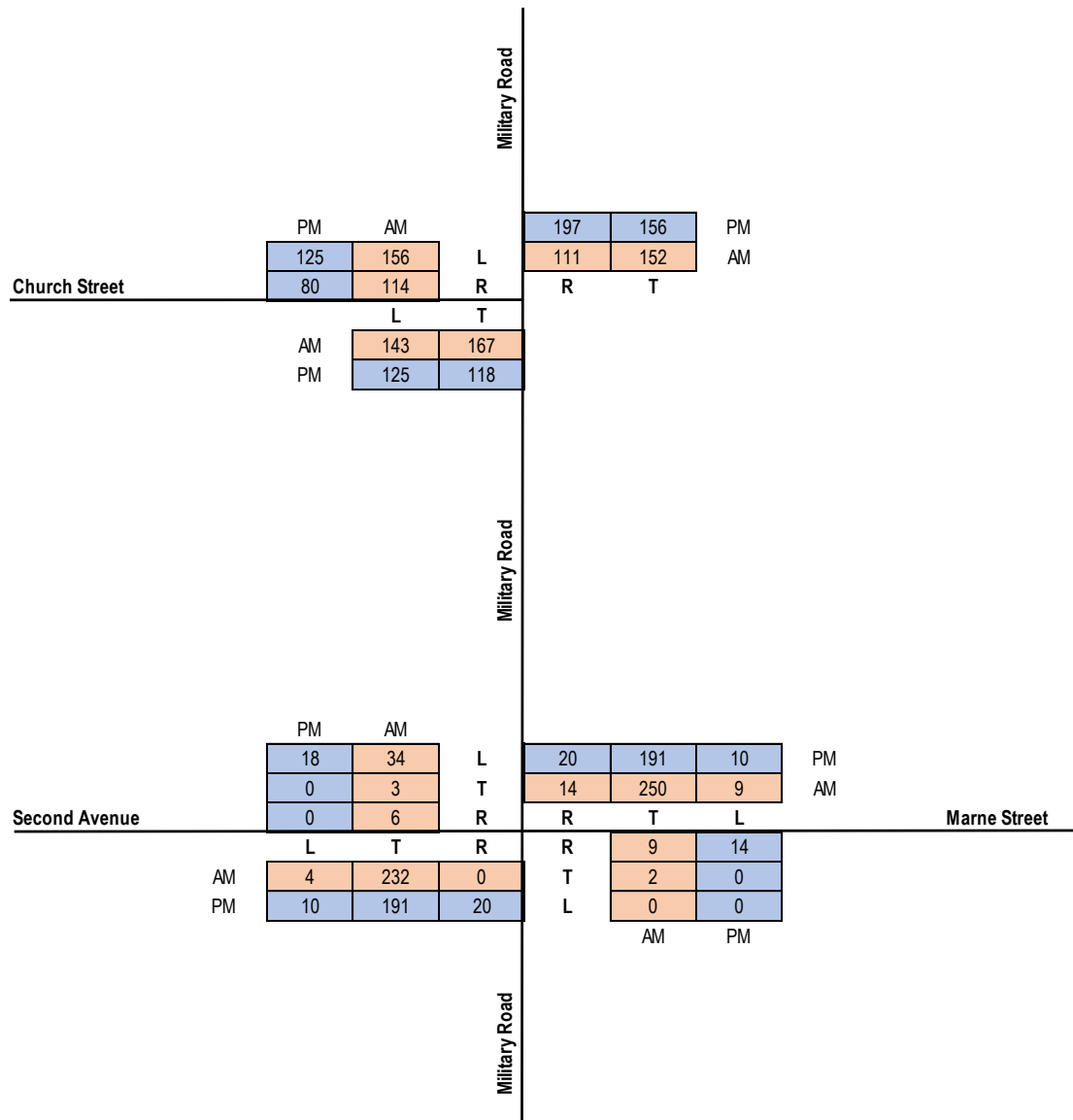


Figure 3.3: 2029 Forecast Background Traffic

3.3 TRAFFIC GENERATION

Traffic generation rates applicable to the proposed development are outlined in the *RMS Guide to Traffic Generating Developments (2002)* and the *Guide to Traffic Generating Developments Technical Direction 04a (2013)*. Maximum traffic generation rates were conservatively adopted due to the relatively low levels of public transport services available. These traffic generation rates and associated AM and PM vehicle trips are outlined in Table 3.1.

Table 3.1: Development Traffic Generation

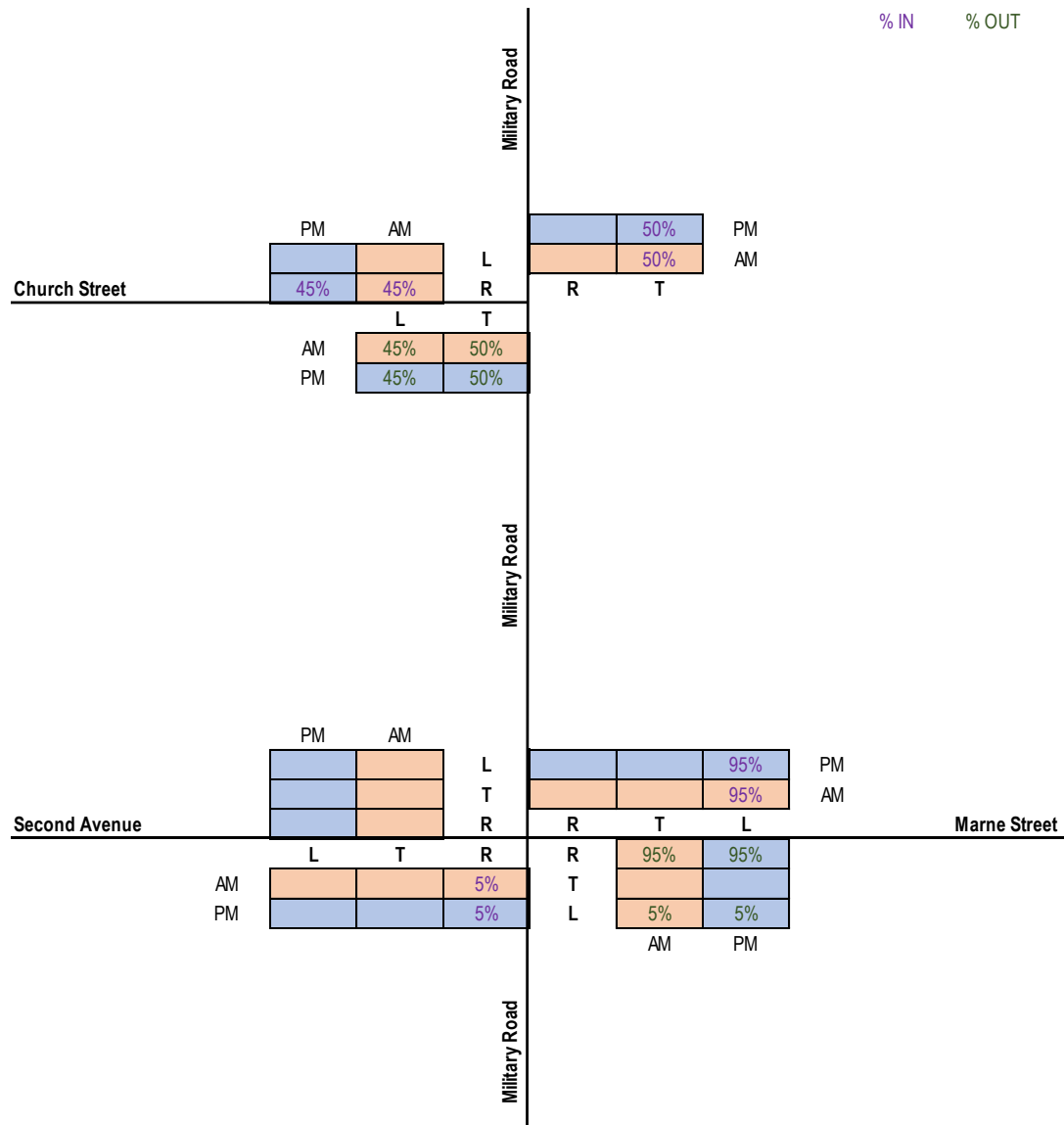
| Land Use | Quantity | AM Rate | PM Rate | AM Trips | PM Trips |
|--------------------------|------------|---------|---------|-----------|-----------|
| Flats up to 2 bedrooms | 36 | 0.5 | 0.5 | 18 | 18 |
| Units 3 or more bedrooms | 64 | 0.65 | 0.65 | 42 | 42 |
| Small lot housing | 7 | 0.85 | 0.9 | 6 | 7 |
| Total | 107 | | | 66 | 67 |

As shown in Table 3.1, the development is anticipated to generate 66 vehicle trips in the AM peak and 67 trips in the PM peak. The peak AM and PM trip splits for the development are shown in Table 3.2.

Table 3.2: Traffic Directionality

| Land Use | AM Trips | PM Trips | AM IN/ OUT % | PM IN/ OUT % | AM Trips IN | AM Trips Out | PM Trips IN | PM Trips OUT |
|-------------------|----------|----------|-----------------|-----------------|-------------|--------------|-------------|--------------|
| Total Residential | 66 | 67 | 20%/80% | 70%/30% | 14 | 53 | 47 | 21 |

The proposed traffic distributions have been derived based on the traffic survey results and taking into consideration the development's expected local catchment. This distribution is shown in Figure 3.4.


Figure 3.4: Development Traffic Distribution

Estimated development traffic volumes are provided in Figure 3.5.



Figure 3.5: Development Traffic

3.4 DESIGN TRAFFIC VOLUMES

The design traffic volumes (i.e. base traffic plus development traffic) for 2019 and 2029 are provided in Figure 3.6 and Figure 3.7 respectively.

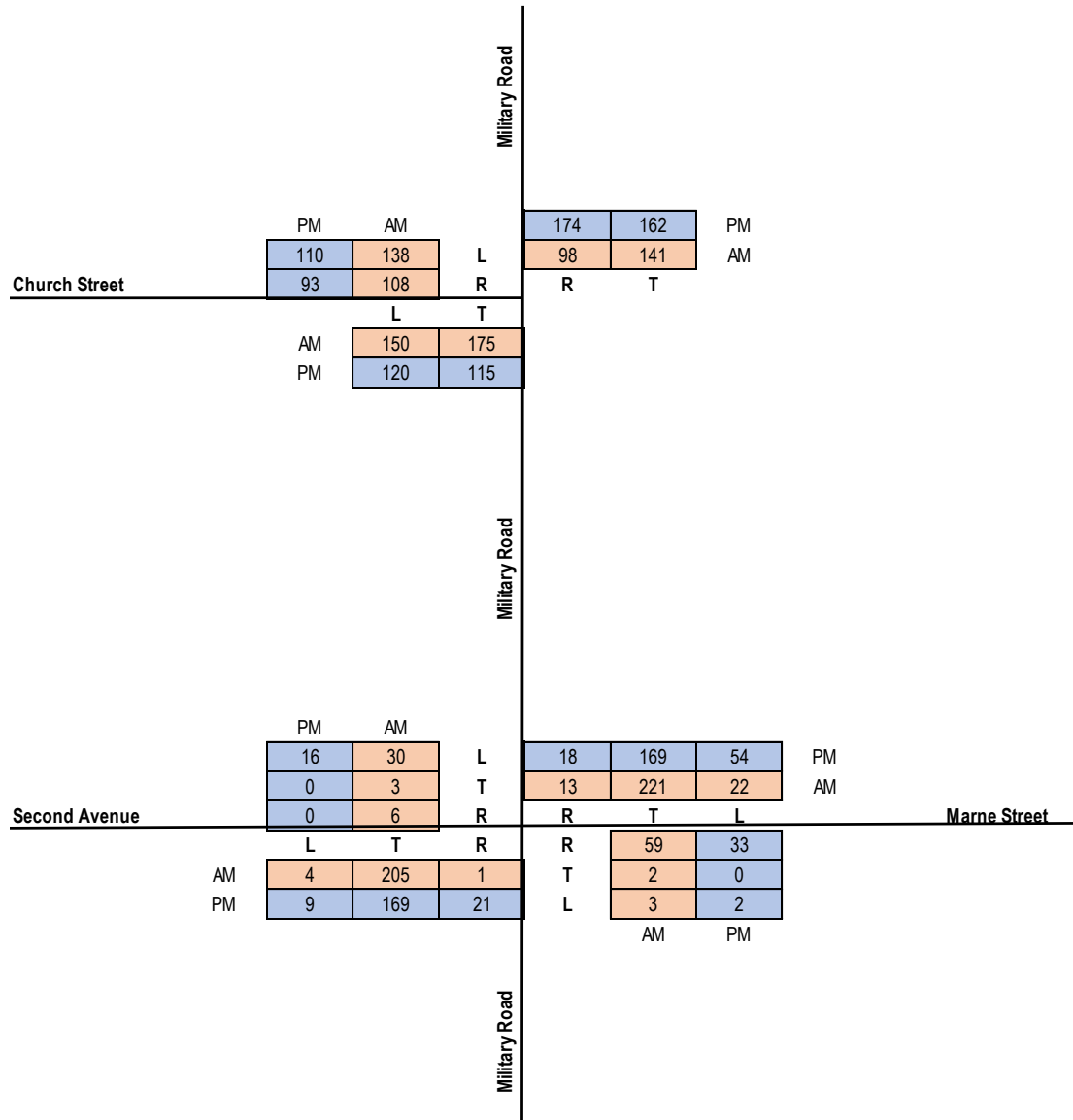


Figure 3.6: 2019 Design Traffic Volumes

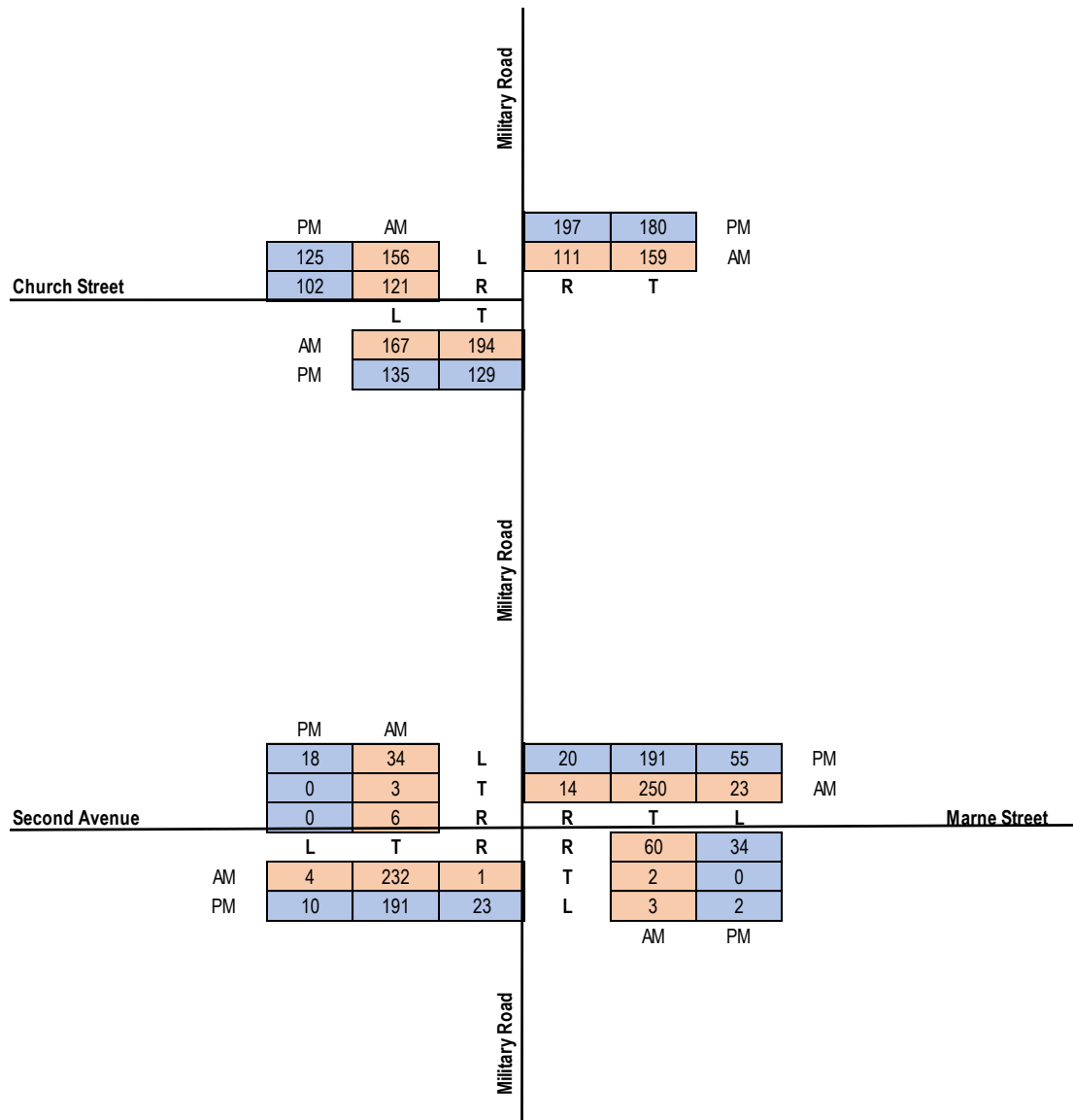


Figure 3.7: 2029 Design Traffic Volumes

3.5 SIDRA ANALYSIS

3.5.1 Methodology

SIDRA Intersection 7.0 Plus was used to model the impact of background and design traffic on the surrounding intersections for the year of opening (2019) and 10-year design horizon (2029). The intersections analysed are as follows:

- Military Road / Marne Street intersection; and
- Military Road / Church Street intersection.

3.5.2 Military Road / Marne Street Intersection

The Military Road / Marne Street intersection as assessed in SIDRA is shown in Figure 3.8.

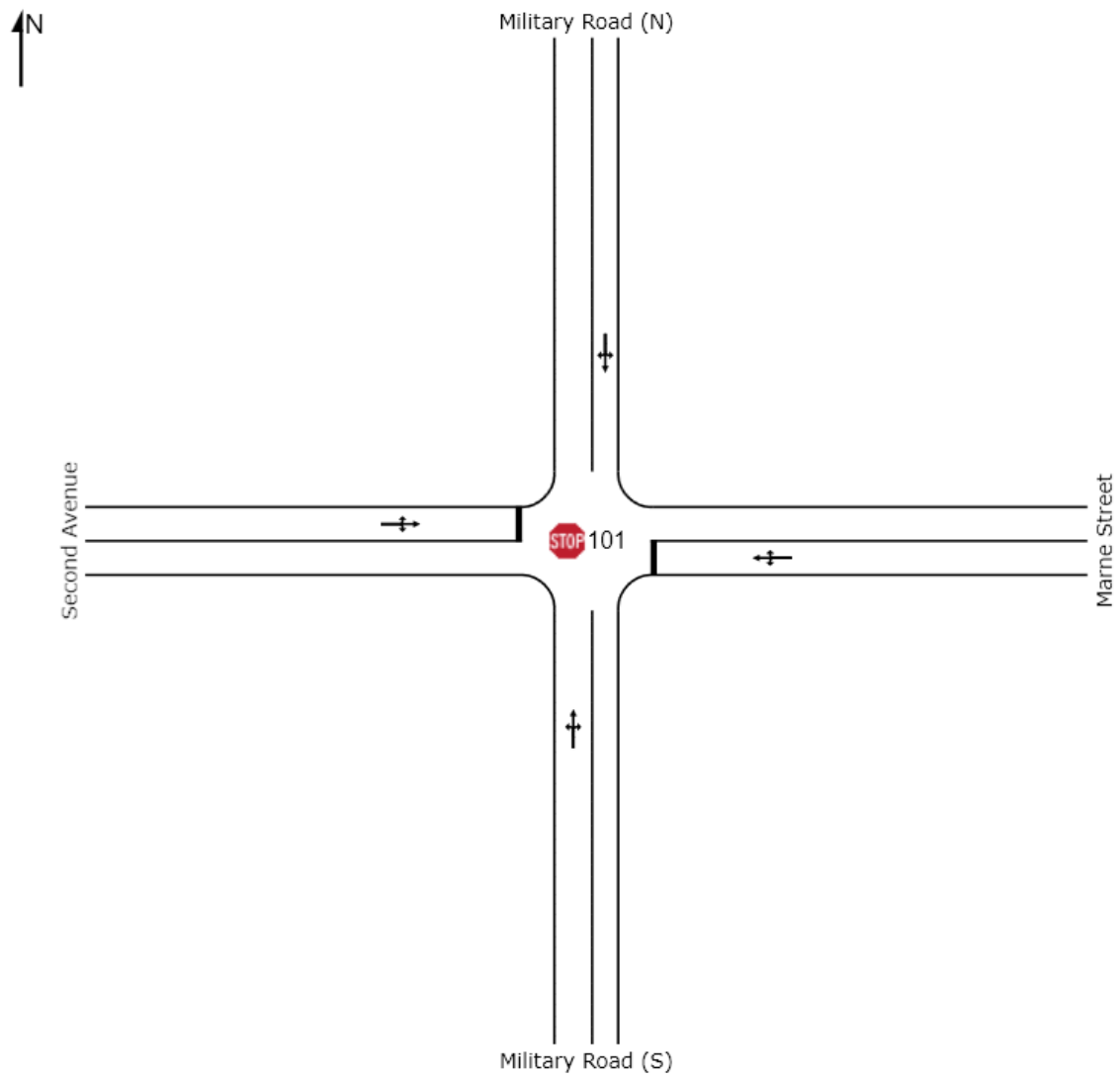


Figure 3.8: Military Road / Marne Street Intersection Layout

The SIDRA Intersection network results for the Military Road / Marne Street for the background traffic volumes are shown in Table 3.3 for years 2019 and 2029.

Table 3.3: Military Road / Marne Street Background SIDRA Results

| Intersection | Movement | AM Peak | | | | PM Peak | | | |
|-------------------|------------|---------|----------------|------------------|------|---------|----------------|------------------|------|
| | | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS |
| 2019 Background | | | | | | | | | |
| Military Road (S) | Left Turn | A | 6 | 0 | 0.11 | A | 6 | 2 | 0.11 |
| | Through | A | 0 | 0 | 0.11 | A | 1 | 2 | 0.11 |
| | Right Turn | A | 7 | 0 | 0.11 | A | 7 | 2 | 0.11 |
| Marne Street | Left Turn | A | 9 | 1 | 0.02 | A | 9 | 1 | 0.02 |
| | Through | A | 11 | 1 | 0.02 | A | 10 | 1 | 0.02 |
| | Right Turn | A | 12 | 1 | 0.02 | A | 11 | 1 | 0.02 |
| Military Road (N) | Left Turn | A | 7 | 1 | 0.13 | A | 6 | 2 | 0.11 |
| | Through | A | 1 | 1 | 0.13 | A | 1 | 2 | 0.11 |
| | Right Turn | A | 7 | 1 | 0.13 | A | 7 | 2 | 0.11 |
| Second Avenue | Left Turn | A | 9 | 2 | 0.04 | A | 9 | 1 | 0.02 |
| | Through | A | 11 | 2 | 0.04 | A | 10 | 1 | 0.02 |
| | Right Turn | A | 12 | 2 | 0.04 | A | 11 | 1 | 0.02 |
| 2029 Background | | | | | | | | | |
| Military Road (S) | Left Turn | A | 6 | 1 | 0.12 | A | 7 | 2 | 0.12 |
| | Through | A | 0 | 1 | 0.12 | A | 1 | 2 | 0.12 |
| | Right Turn | A | 7 | 1 | 0.12 | A | 7 | 2 | 0.12 |
| Marne Street | Left Turn | A | 9 | 1 | 0.02 | A | 9 | 1 | 0.03 |
| | Through | A | 11 | 1 | 0.02 | A | 11 | 1 | 0.03 |
| | Right Turn | A | 12 | 1 | 0.02 | A | 12 | 1 | 0.03 |
| Military Road (N) | Left Turn | A | 7 | 1 | 0.15 | A | 7 | 2 | 0.13 |
| | Through | A | 1 | 1 | 0.15 | A | 1 | 2 | 0.13 |
| | Right Turn | A | 7 | 1 | 0.15 | A | 7 | 2 | 0.13 |
| Second Avenue | Left Turn | A | 9 | 2 | 0.05 | A | 9 | 1 | 0.02 |
| | Through | A | 11 | 2 | 0.05 | A | 11 | 1 | 0.02 |
| | Right Turn | A | 12 | 2 | 0.05 | A | 11 | 1 | 0.02 |

Table 3.3 demonstrates that the intersection of Military Road / Marne Street is expected to operate within acceptable performance limits for a priority-controlled intersection with 2019 and 2029 background traffic volumes.

The SIDRA Intersection network results for the Military Road / Marne Street for the design traffic volumes are shown in Table 3.4 for 2029.

Table 3.4: Military Road / Marne Street Design SIDRA Results

| Intersection | Movement | AM Peak | | | | PM Peak | | | |
|-------------------|------------|---------|----------------|------------------|------|---------|----------------|------------------|------|
| | | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS |
| 2029 Design | | | | | | | | | |
| Military Road (S) | Left Turn | A | 6 | 1 | 0.12 | A | 7 | 2 | 0.12 |
| | Through | A | 0 | 1 | 0.12 | A | 1 | 2 | 0.12 |
| | Right Turn | A | 7 | 1 | 0.12 | A | 7 | 2 | 0.12 |
| Marne Street | Left Turn | A | 10 | 4 | 0.13 | A | 9 | 2 | 0.07 |
| | Through | A | 12 | 4 | 0.13 | A | 11 | 2 | 0.07 |
| | Right Turn | A | 13 | 4 | 0.13 | A | 12 | 2 | 0.07 |
| Military Road (N) | Left Turn | A | 6 | 2 | 0.16 | A | 6 | 2 | 0.15 |
| | Through | A | 1 | 2 | 0.16 | A | 1 | 2 | 0.15 |
| | Right Turn | A | 7 | 2 | 0.16 | A | 7 | 2 | 0.15 |
| Second Avenue | Left Turn | A | 9 | 2 | 0.5 | A | 9 | 1 | 0.02 |
| | Through | A | 12 | 2 | 0.5 | A | 11 | 1 | 0.02 |
| | Right Turn | A | 12 | 2 | 0.5 | A | 11 | 1 | 0.02 |

Table 3.4 demonstrates that the intersection of Military Road / Marne Street is expected to operate within acceptable performance limits for a priority-controlled intersection with 2029 design traffic volumes.

3.5.3 Military Road / Church Street Intersection

The Military Road / Church Street intersection as assessed in SIDRA is shown in Figure 3.9.

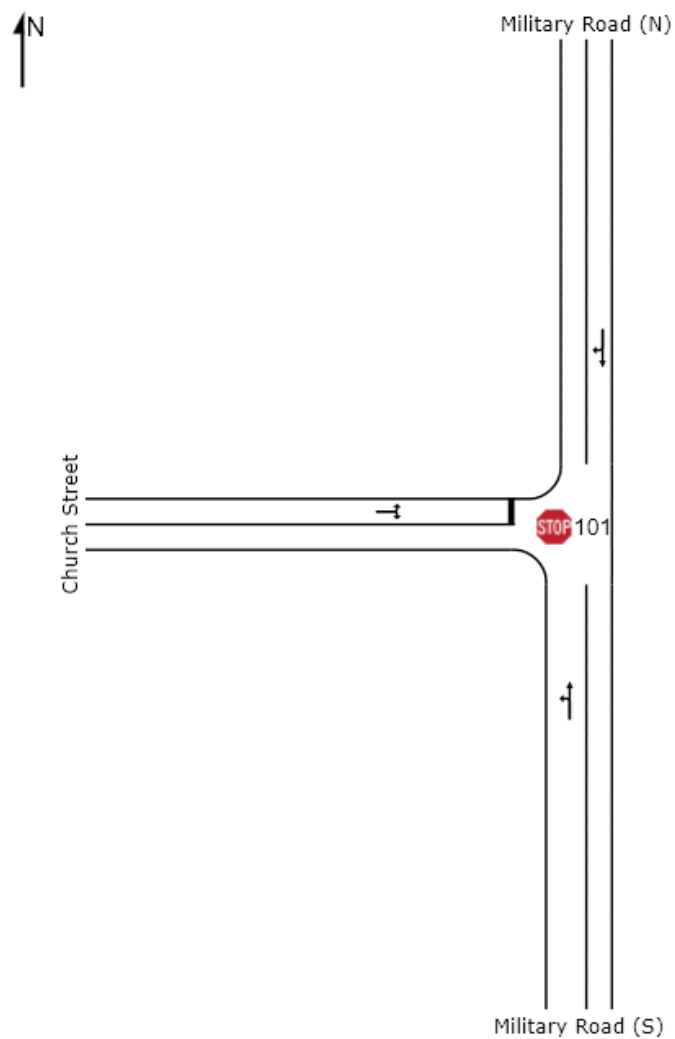


Figure 3.9: Military Road / Church Street Intersection Layout

The SIDRA Intersection network results for the Military Road / Church Street intersection layout for the background traffic volumes are shown in Table 3.5 for years 2019 and 2029.

Table 3.5: Military Road / Church Street Existing Layout Background SIDRA Results

| Intersection | Movement | AM Peak | | | | PM Peak | | | |
|-------------------|------------|---------|----------------|------------------|------|---------|----------------|------------------|------|
| | | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS |
| 2019 Background | | | | | | | | | |
| Military Road (S) | Left Turn | A | 5 | 0 | 0.14 | A | 5 | 0 | 0.11 |
| | Through | A | 0 | 0 | 0.14 | A | 0 | 0 | 0.11 |
| Military Road (N) | Through | A | 1 | 5 | 0.16 | A | 1 | 9 | 0.21 |
| | Right Turn | A | 7 | 5 | 0.16 | A | 6 | 9 | 0.21 |
| Church Street | Left Turn | A | 9 | 11 | 0.28 | A | 9 | 6 | 0.21 |
| | Right Turn | A | 12 | 11 | 0.28 | A | 12 | 6 | 0.21 |
| 2029 Background | | | | | | | | | |
| Military Road (S) | Left Turn | A | 5 | 0 | 0.16 | A | 5 | 0 | 0.13 |
| | Through | A | 0 | 0 | 0.16 | A | 0 | 0 | 0.13 |
| Military Road (N) | Through | A | 1 | 7 | 0.18 | A | 1 | 10 | 0.24 |
| | Right Turn | A | 7 | 7 | 0.18 | A | 7 | 10 | 0.24 |
| Church Street | Left Turn | A | 10 | 11 | 0.33 | A | 9 | 8 | 0.25 |
| | Right Turn | A | 12 | 11 | 0.33 | A | 13 | 8 | 0.25 |

Table 3.5 demonstrates that the intersection of Military Road / Church Street is expected to operate within acceptable performance limits for a priority-controlled intersection with 2019 and 2029 background traffic volumes.

The SIDRA Intersection network results for the Military Road / Church Street intersection layout for the design traffic volumes are shown in Table 3.6 for 2029.

Table 3.6: Military Road / Church Street Existing Layout Design SIDRA Results

| Intersection | Movement | AM Peak | | | | PM Peak | | | |
|-------------------|------------|---------|----------------|------------------|------|---------|----------------|------------------|------|
| | | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS | LOS | Avg. Delay (s) | 95%ile Queue (m) | DOS |
| 2029 Design | | | | | | | | | |
| Military Road (S) | Left Turn | A | 5 | 0 | 0.19 | A | 5 | 0 | 0.14 |
| | Through | A | 0 | 0 | 0.19 | A | 0 | 0 | 0.14 |
| Military Road (N) | Through | A | 2 | 7 | 0.19 | A | 1 | 11 | 0.26 |
| | Right Turn | A | 7 | 7 | 0.19 | A | 7 | 11 | 0.26 |
| Church Street | Left Turn | A | 10 | 13 | 0.36 | A | 9 | 9 | 0.30 |
| | Right Turn | A | 12 | 13 | 0.36 | A | 13 | 9 | 0.30 |

Table 3.6 demonstrates that the intersection of Military Road / Church Street is expected to operate within acceptable performance limits for a priority-controlled intersection with 2029 design traffic volumes.

4. PARKING ASSESSMENT

4.1 CAR PARKING REQUIREMENTS

The relevant car parking requirements are stipulated in the Wollongong City Council *Development Control Plan (DCP) – Chapter E3: Car Parking, Access, Servicing / Loading Facilities and Traffic Management*. Table 4.1 details the applicable car parking rates and car parking requirements. DCP parking rates are determined based on the floor area of each dwelling. As the proposed floor area is not defined at the time of this report (February 2018) the following conservative assumptions were made:

- 1-bedroom flat floor area is less than 70m²;
- 2-bedroom flat floor area is greater than 70m² but less than 110m²;
- 3-bedroom townhouse floor area is greater than 110m²; and
- small lot dwellings are greater than 125m².

Table 4.1: Car Parking Requirement

| Land Use | Type | Parking Rate | Quantity | Parking Spaces Required |
|--|---|-------------------------|----------|-------------------------|
| Dwelling House | Resident parking >125m ² dwelling | 2 spaces per dwelling | 7 | 14 |
| Residential Flat Building / Multi-Dwelling Housing / Attached Dwelling | Resident parking <70m ² dwelling | 1 space per dwelling | 12 | 12 |
| | Resident parking 70-110m ² dwelling | 1.5 spaces per dwelling | 24 | 36 |
| | Resident parking >110m ² dwelling | 2 spaces per dwelling | 64 | 128 |
| | Visitor parking | 0.2 spaces per dwelling | 100 | 20 |
| Total | | | | 210 |

As shown in Table 4.1, a total of 210 car spaces are required across the development site. It is noted that double garages are proposed with each small lot house in accordance with DCP requirements. As such, 176 resident car parking spaces and 20 visitor car parking spaces shall be provided to service the corresponding terraces / townhouses and flats. Adaptable car parking spaces shall also be provided as required to meet adaptable housing requirements outlined in the DCP.

4.2 BICYCLE / MOTORCYCLE PARKING REQUIREMENTS

Dedicated motorcycle and bicycle parking facilities are required to be provided for residents living in the residential flats and 2-3 story terrace style residential. Rates determining these parking requirements are stipulated in the Wollongong City Council *Development Control Plan (DCP) – Chapter E3: Car Parking, Access, Servicing / Loading Facilities and Traffic Management*. Table 4.2 details the applicable bicycle and motorcycle parking rates and relevant parking requirements based upon development yields.

Table 4.2: Bicycle & Motorcycle Parking Requirement

| Parking | Type | Parking Rate | Quantity | Spaces Required |
|------------|-------------------------------------|--------------------|----------|-----------------|
| Motorcycle | Resident Parking | 1 per 15 dwellings | 100 | 7 |
| Bicycle | Resident Parking (Security Class B) | 1 per 3 dwellings | | 34 |
| | Visitor Parking (Security Class C) | 1 per 12 dwellings | | 9 |

4.3 PARKING LAYOUT

The on-site parking is to be designed in accordance with the relevant requirements of Council's DCP and Australian Standards AS2890. The proposed development shall comply with the following:

- resident car parking bay are to be 2.4m wide by 5.4m long (user class 1A);
- visitor car parking bays are to be 2.6m wide by 5.4m long (user class 3);
- car parking aisle widths are to be 5.8m wide plus an additional 0.3m clearance to vertical obstructions;
- double garages are to have a 6m internal width and length;
- domestic driveway grades are to have a maximum grade of 1:20 (5%) across the verge, and a maximum grade of 1:4(25%) elsewhere;
- domestic driveway width is to be a minimum of 3m located at least 6m from any intersections;
- access ramps are to have a maximum of 1:5 (20%) with 2m grade transitions if used only by cars. If service vehicles between the size of a MRV and HRV are to use the access ramp, maximum grade is to be 1:5.6 (15.4%) with a maximum rate of change of grade of 1:16 (6.25%) in 7m of travel;
- accesses are to be a minimum of 6m wide; and
- bicycle parking is to be a minimum of 0.5m wide and 1.8m long with a minimum aisle width of 1.5m

5. INTERNAL LAYOUT AND ACCESS

5.1 ELECTROLYTIC STREET

The existing Electrolytic Street road width is less than the width specified for an 'Access Street' road type in accordance with the Wollongong DCP and as such isn't sufficient to cater for two-way traffic flow. The existing road width is sufficient for a one-way 'Access Place'. Due to road gradient, intersecting angle and other vertical obstructions, vehicles exiting Electrolytic Street onto Military Road have poor available sight distance. It is therefore recommended that Electrolytic Street be restricted to one-way, eastbound traffic flow, with vehicles entering from Military Road.

In accordance with requirements outlined in AS1742, signage and line marking shall be provided as required to Electrolytic Street to one-way traffic movements only. Provision of a concrete blister is also recommended at the end of Reservoir Street to further discourage any northbound vehicles from turning left into Electrolytic Street. The location of the signage, line marking, and the recommended blister is detailed in Appendix D.

5.2 ACCESSES

Access to the seven (7) small-lot dwellings is proposed as individual driveways for each dwelling off Marne Street. Two (2) accesses are proposed for the townhouse / terrace style accommodation off Reservoir Street. It is recommended that the northern access to the townhouses off Reservoir Street be relocated to be a minimum of 45m south from the Reservoir Street / Electrolytic Street intersection. The residential flats to the north of the site will be accessed via Electrolytic Street. Figure 5.1 shows the proposed location of all site access points.



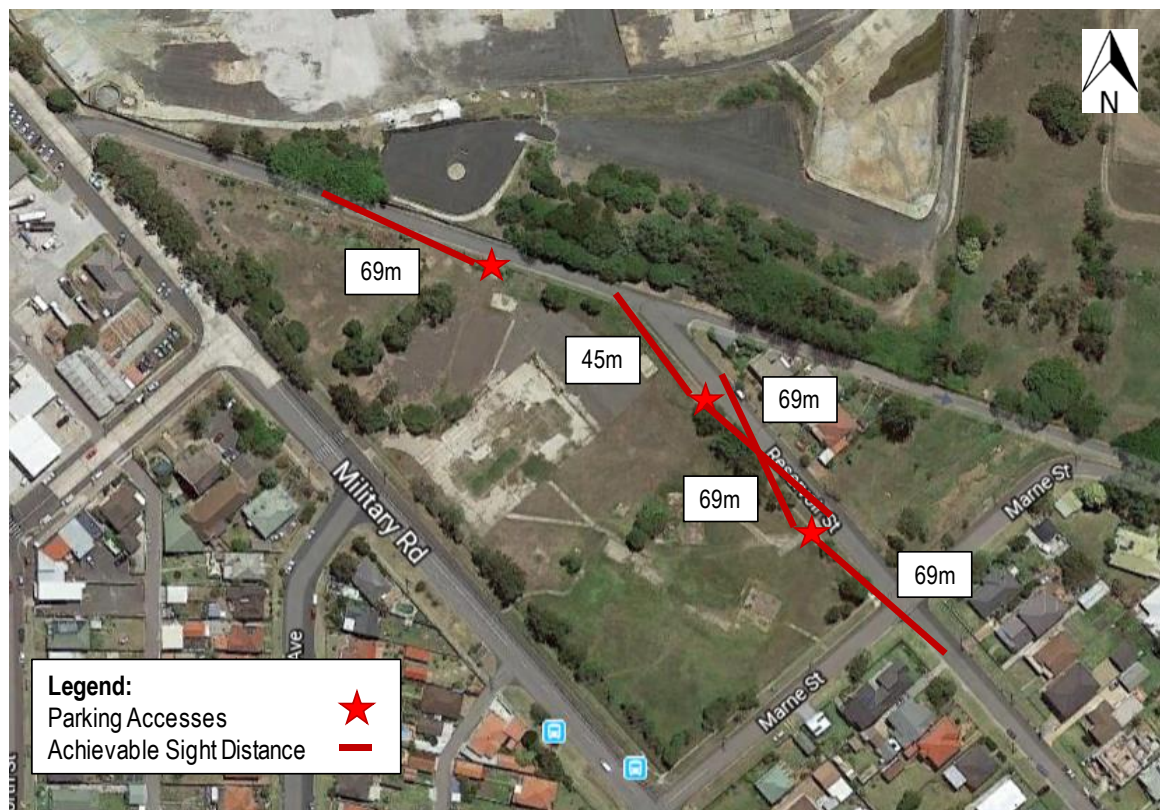
Figure 5.1: Site Access Locations

5.3 SERVICING & REFUSE COLLECTION

Kerbside refuse collection will be undertaken for the proposed small-lot dwellings. It is expected that refuse collection for all other dwellings will be undertaken on-site by a front-loading refuse collection vehicle (RCV). As such, the internal layout shall allow for a 10.24m RCV to manoeuvre within the site and ingress and egress in forward gear. The design service vehicle for the site is an 8.8m medium rigid vehicle (MRV). Therefore, as the internal layout shall facilitate a 10.24m RCV, the layout shall also facilitate an 8.8m MRV. MRV service bays shall be provided as required to service the proposed development.

5.4 SIGHT DISTANCE

A sight distance assessment was conducted for the proposed accesses to the townhouse / terrace style housing and residential flats. Considering a speed limit of 50km/h, in accordance with AS2890.1, desirable sight distance is 69m. Achievable sight distance for each access are shown in Figure 5.2, noting that adequate sight distance is only required west of the Electrolytic Street access if restricted to one-way traffic flow.



Source: Google Earth

Figure 5.2: Achievable Sight Distances

The desired 69m sight distance is not achievable from the relocated northern access recommended off Reservoir Street. However, minimum sight distance in accordance with AS2890.1 is achieved. It is also noted that the distance to the Marne Street / Reservoir Street intersection is less than the required sight distance. However, vehicles turning into Reservoir Street from Marne Street or Electrolytic Street are expected to slow below 50km/h. There is also very minimal development on Reservoir Street north of Marne Street and as such conflicting traffic will be minimal. Therefore, safety is not deemed to be adversely impacted.

6. ALTERNATIVE TRANSPORT MODES

6.1 PUBLIC TRANSPORT

Two (2) existing bus stops are present on Military Road fronting the development immediately north of the Military Road / Marne Street intersection. It is recommended that the two bus stops (Stop IDs 250544) be upgraded to formalised bus shelters designed in accordance with requirements outlined in AS1428.1: *Design for Access and Mobility*. These bus stops service Route 43 and Route 65 providing public transport connectivity from Port Kembla to Dapto and the Wollongong CBD.

While it is noted that heavy rail services are available, as service frequency is low and the distance from the proposed site to the train station is significant, no allowances for development connectivity to the rail network are required.

6.2 ACTIVE TRANSPORT

The proposed development provides dedicated pedestrian links through the site and pedestrian site access points on Military Road, Reservoir Street and Electrolytic Street. Accesses to the proposed green zone at the northern end of the site are also proposed. It is recommended that further pedestrian facilities be provided to facilitate connectivity between the site and the Port Kembla Town Centre and the existing active transport network northwest of the site. Allowances should also be made to provide safe and efficient access to the bus stops on Military Road adjacent to the site.

It is recommended that the existing footpath on Reservoir Street be extended to connect with the proposed pedestrian access off Electrolytic Street. In accordance with the Wollongong DCP, verge cross-fall is to be a maximum of 2.5% from the back of kerb to the property boundary. As such, in order to achieve this on Military Road, it is recommended that pedestrian stairs fronting the site are removed, earthworks are conducted as required and a retaining wall, stairs / ramps and pedestrian protection be provided. A 2.5m shared path is recommended on the upgraded Military Road verge fronting the site. Figure 6.1 details the location of these recommendations.



Figure 6.1: Active Transport Recommendations

The existing pedestrian crossing on Military Road between Church Street and Third Avenue is non-compliant with RMS and Australian Standards. As such, three (3) recommended options have been identified to provide a suitable pedestrian treatment for active transport users to cross Military road. The upgrade the existing pedestrian crossing so that the crossing is provided with kerb ramps, line marking and signage compliant with requirements outlined in AS1742.10.

In accordance with the RMS Supplement to AS1742 – Manual of Uniform Traffic Control Devices (2013), the expected pedestrian and traffic volumes on Military Road do not warrant a 'zebra' crossing. As such, an alternative option is to remove the existing pedestrian crossing and instead provide kerb blisters designed in accordance with AS1742.10 at the same location.

A third potential option is to provide a formalised, channelised right-turn (CHR) treatment on the northern approach to the Military Road / Church Street intersection. The provision of this CHR facilitates a pedestrian refuge to be placed opposite, on the southern approach to the Military Road / Church Street intersection. If implemented this pedestrian refuge is to be designed in accordance with all relevant requirements stipulated in AS1742.10.

Pedestrian links from internal footpaths to Military Road shall connect directly with the proposed pedestrian crossing at the north of the site and bus stops to the south of the site. With the development and associated upgrades, there will be high levels of pedestrian amenity and connectivity with nearby bus services and the Port Kembla Town Centre.

7. CONCLUSION

The key findings of the traffic impact assessment for the proposed medium-density residential development located at 114-126 Military Road, Port Kembla are as follows:

- the proposed residential development will consist of seven (7) small-lot dwellings, 64 medium density townhouse/terrace style dwellings and 36 residential flats;
- the proposed development is expected to generate 66 vehicle trips in the AM peak hour and 67 vehicle trips in the PM peak hour;
- the existing Military Road / Marne Street and Military Road / Church Street intersections can sufficiently cater for design traffic for the 10-year design horizon;
- in addition to the 14 car spaces proposed for the small lot dwellings, 176 resident car parking spaces and 20 visitor car parking spaces shall be provided in accordance with Chapter E3 of the Wollongong DCP;
- on-site parking is to be designed in accordance with the relevant requirements of Council's DCP and Australian Standards AS2890;
- it is recommended that Electrolytic Street be restricted to eastbound one-way traffic flow with signage and line marking provided as required in accordance with AS1742;
- the internal layout of the site shall allow a 10.24m front-loading RCV to manoeuvre the site and enter and exit in forward gear;
- the site is expected to be serviced by an 8.8m MRV and as such a service bay shall be provided;
- it is recommended that existing bus stops on Military Road fronting the site be upgraded to formalised bus shelters designed in accordance with requirements outlined in *AS1428.1: Design for Access and Mobility*;
- it is recommended that the existing footpath on Reservoir Street be extended to connect with the proposed pedestrian access off Electrolytic Street;
- it is recommended that earthworks be conducted on the Military Road verge fronting the site to provide a maximum 2.5% crossfall. A 2.5m wide shared path is also recommended on this verge in accordance with the Wollongong DCP; and
- as the existing pedestrian crossing on Military Road is non-compliant, it is recommended that it is upgraded or replaced with an alternative pedestrian treatment.

Based on the above assessment we conclude that there are no significant traffic or transport impacts associated with the proposed development to preclude its approval and relevant conditioning on transport planning grounds.

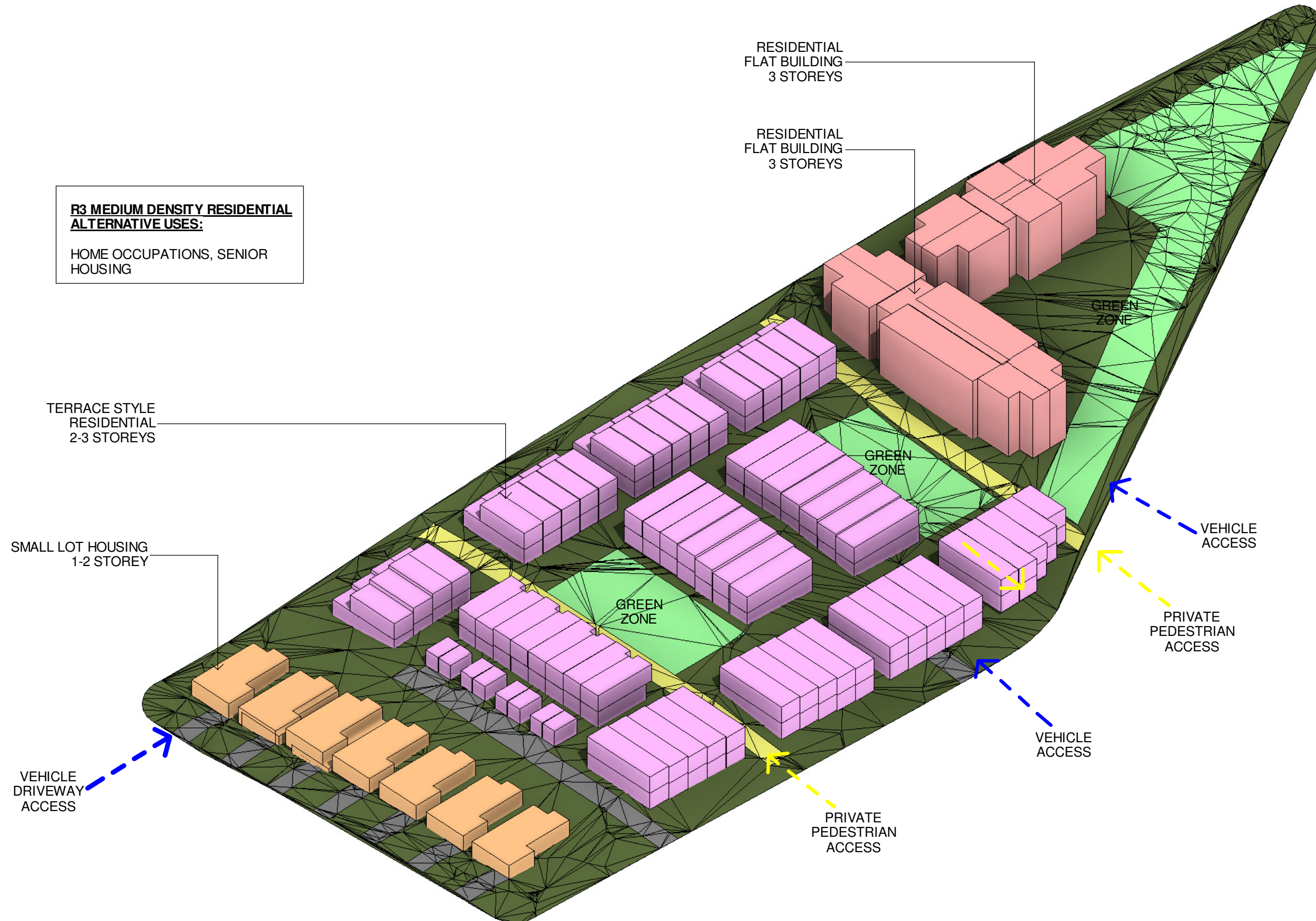
APPENDIX A

DEVELOPMENT PLANS

Proposed Site Development and Context

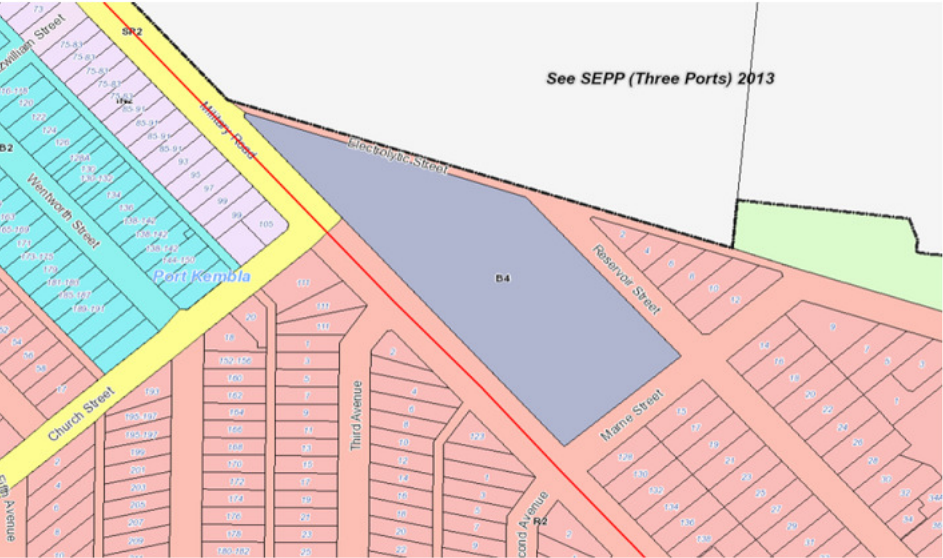


Proposed Site Development - 3d



LEP Maps Proposed Site and Context

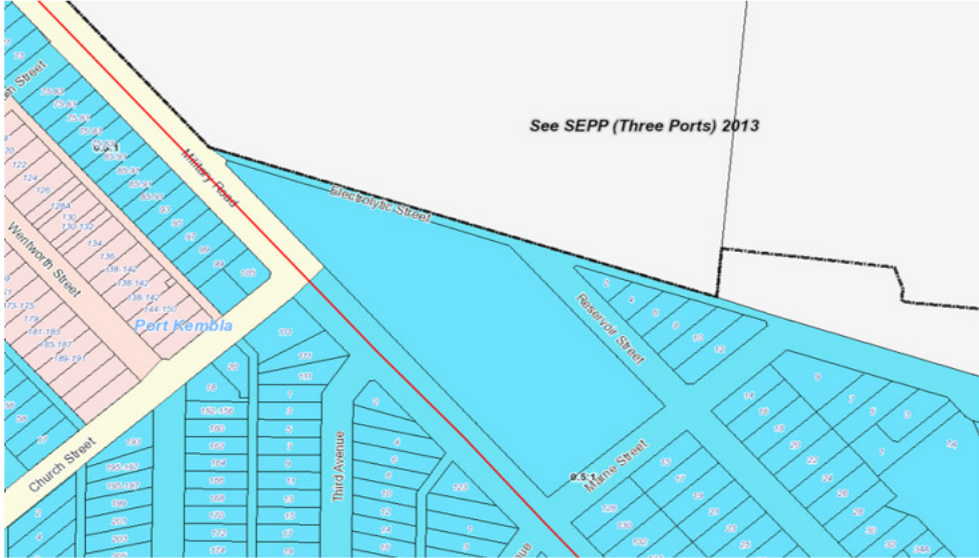
CURRENT WOLLONGONG LEP MAPS



Land Zoning - B4 Mixed Use

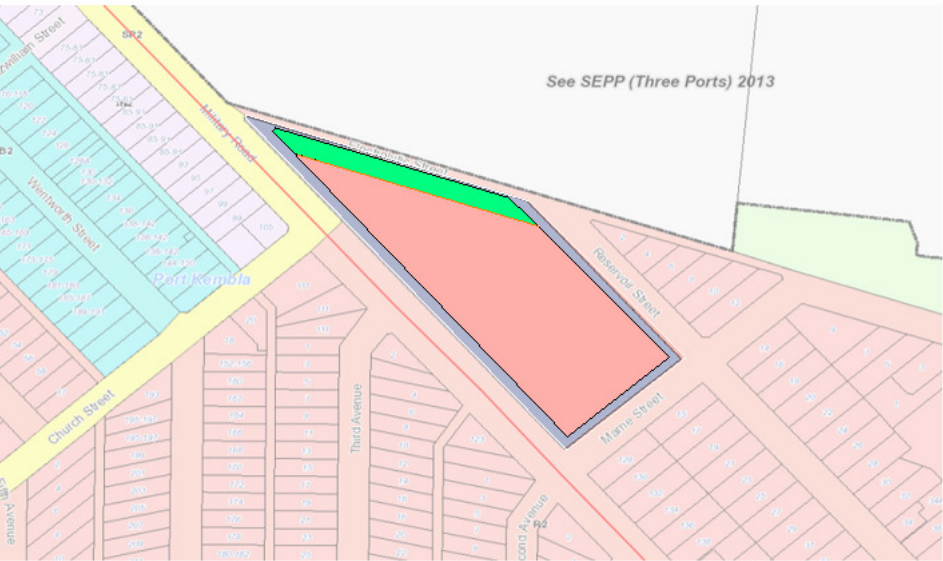


Height of Buildings - 9m

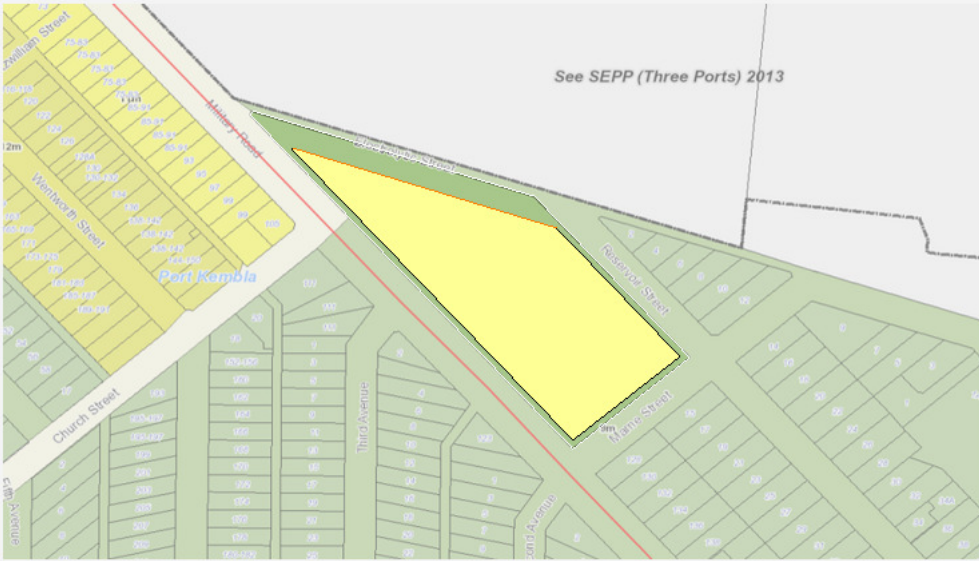


Floor Space Ratio - 0.5 : 1

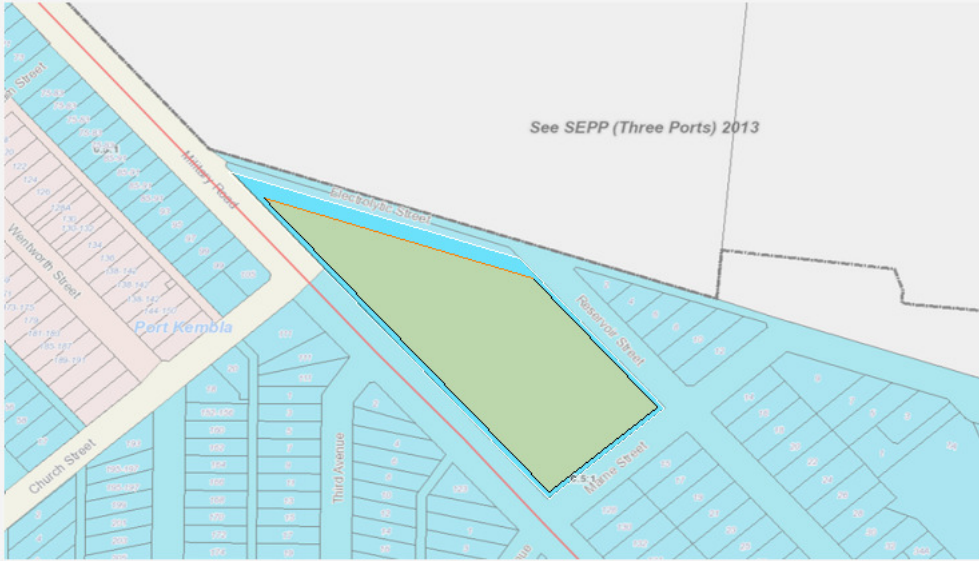
PROPOSED AMENDMENTS TO WOLLONGONG LEP MAPS



R3 (Medium Density Residential) ■
RE2 (Private Recreation) ■

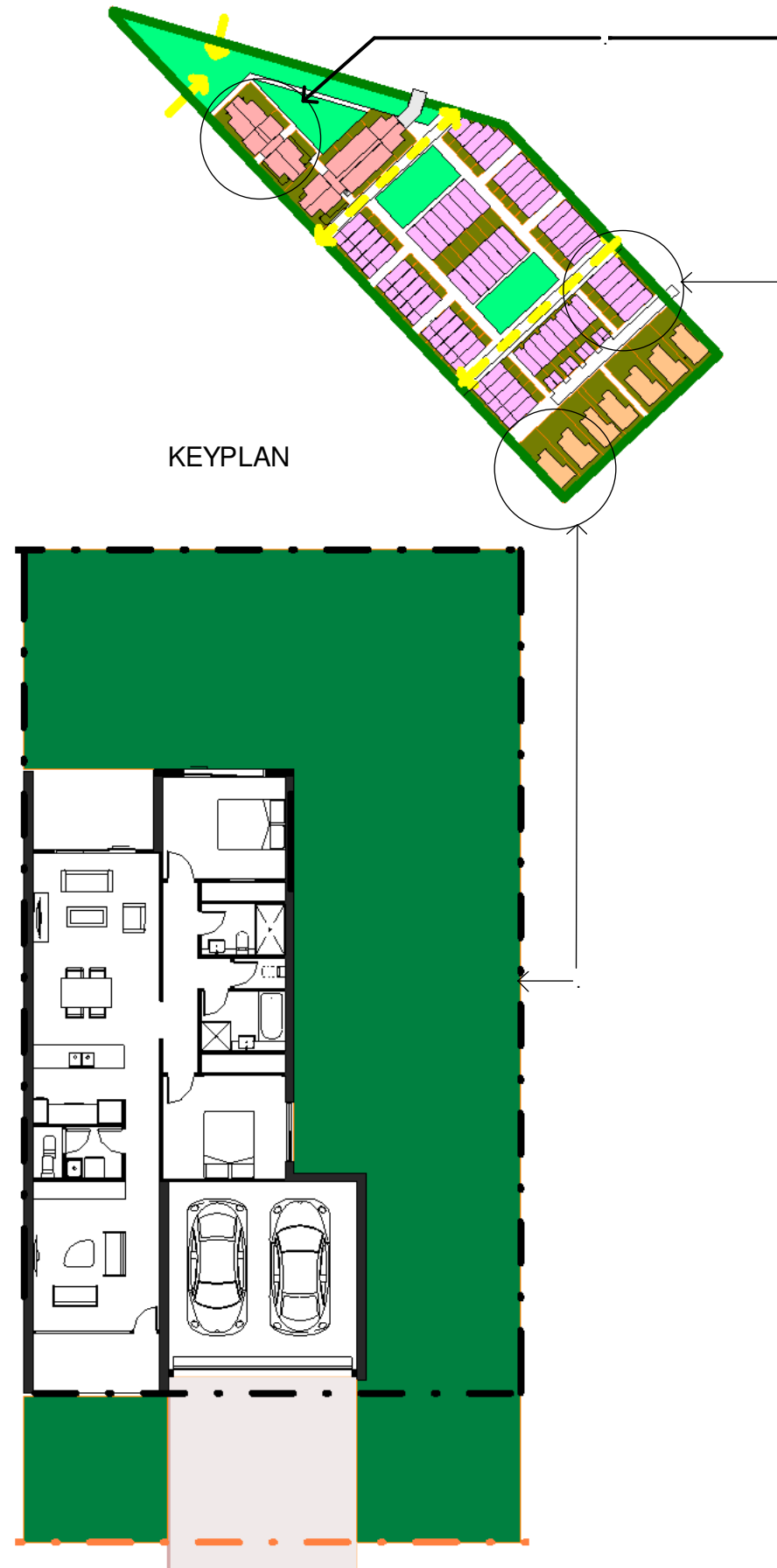


11 METER
HEIGHT LIMIT ■



0.75:1 FSR ■

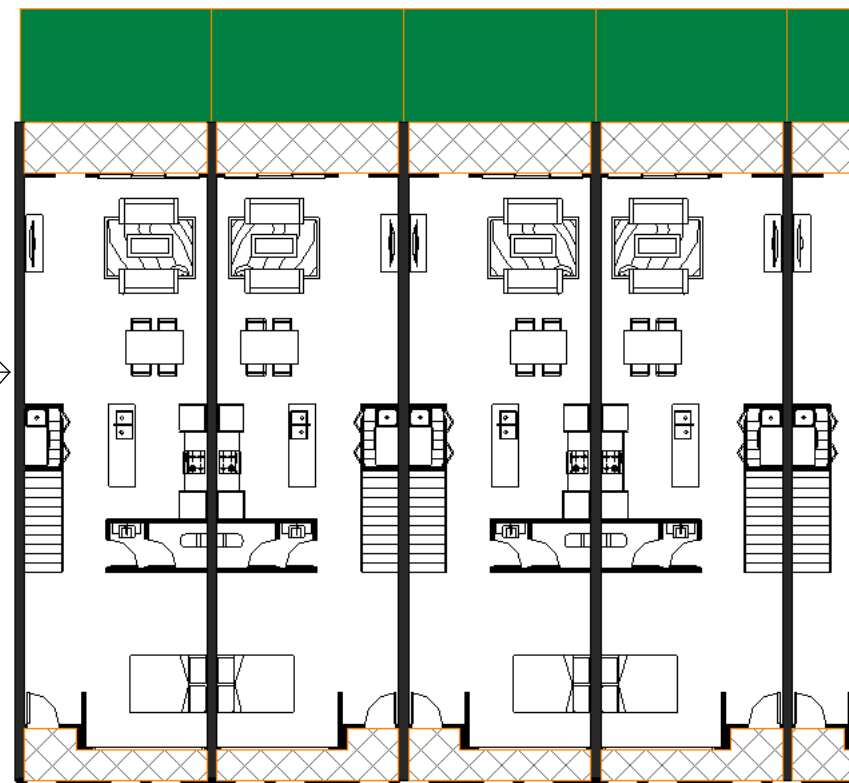
TYPICAL FLOOR PLANS



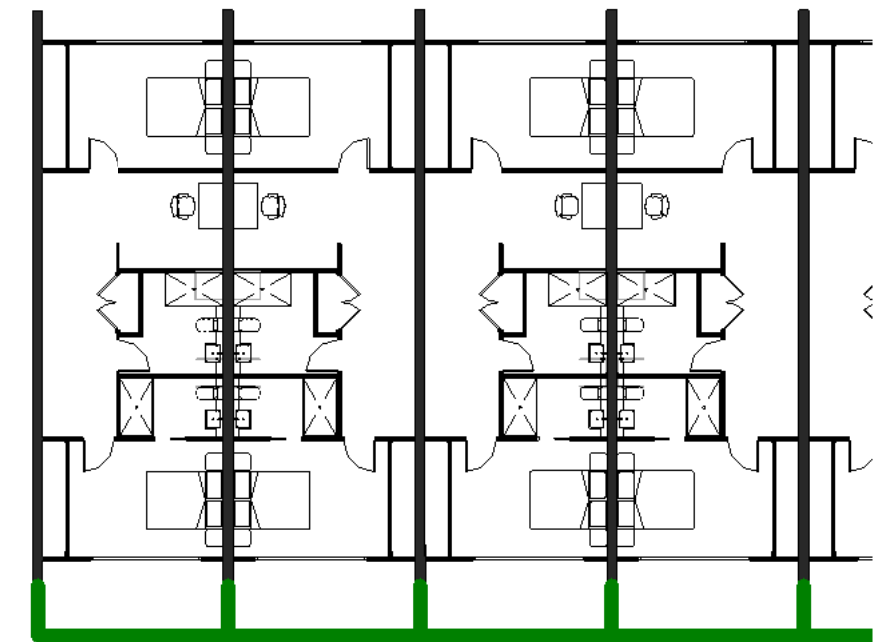
STAGE 1 - TYPICAL SMALL LOT HOUSING



STAGE 3 - TYPICAL FLOOR OF UNIT BLOCK



STAGE 2 - TYPICAL TOWNHOUSES - GROUND FLOOR



STAGE 2 - TYPICAL TOWNHOUSES - UPPER FLOOR

APPENDIX B

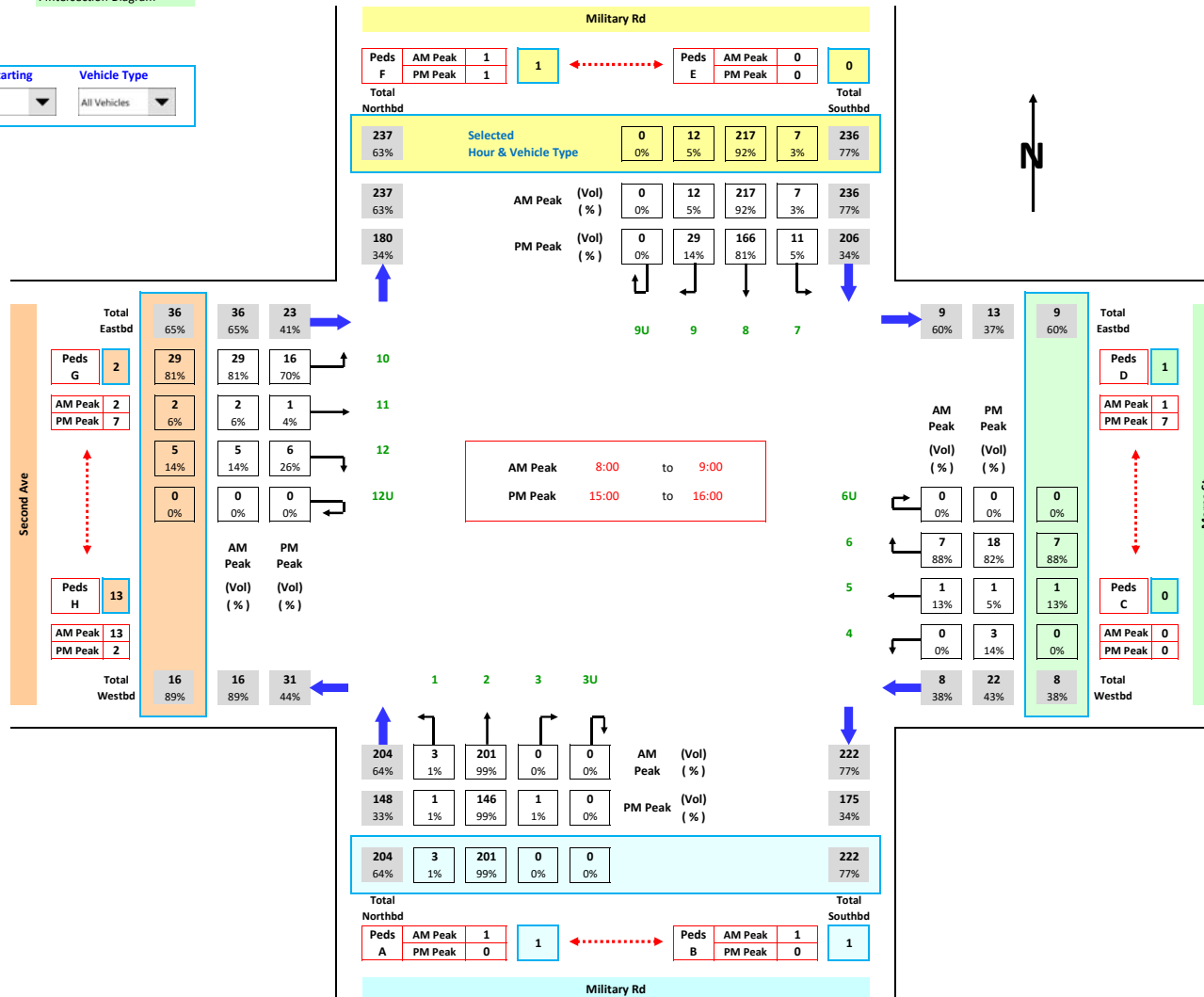
TRAFFIC COUNTS

Job No. : N3855
 Client : Bitzios
 Suburb : Port Kembla
 Location : 1. Military Rd / Marne St / Second Ave

Day/Date : Friday, 2nd February 2018
 Weather : Fine
 Description : Classified Intersection Count
 : Intersection Diagram



Hour Starting Vehicle Type

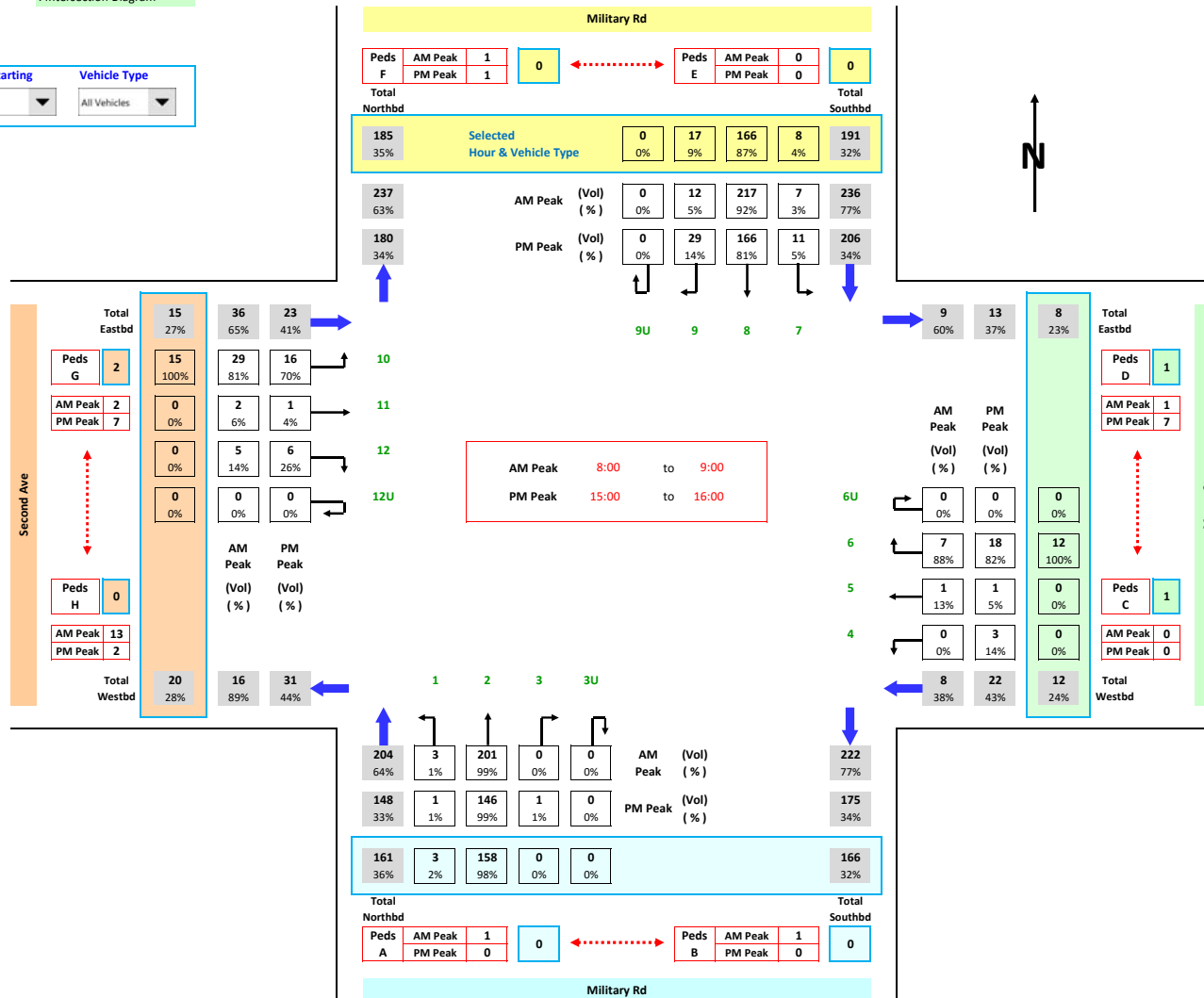


Job No. : N3855
Client : Bitzios
Suburb : Port Kembla
Location : 1. Military Rd / Marne St / Second Ave

Day/Date : Friday, 2nd February 2018
Weather : Fine
Description : Classified Intersection Count
Intersection Diagram



Hour Starting: 15:00
Vehicle Type: All Vehicles

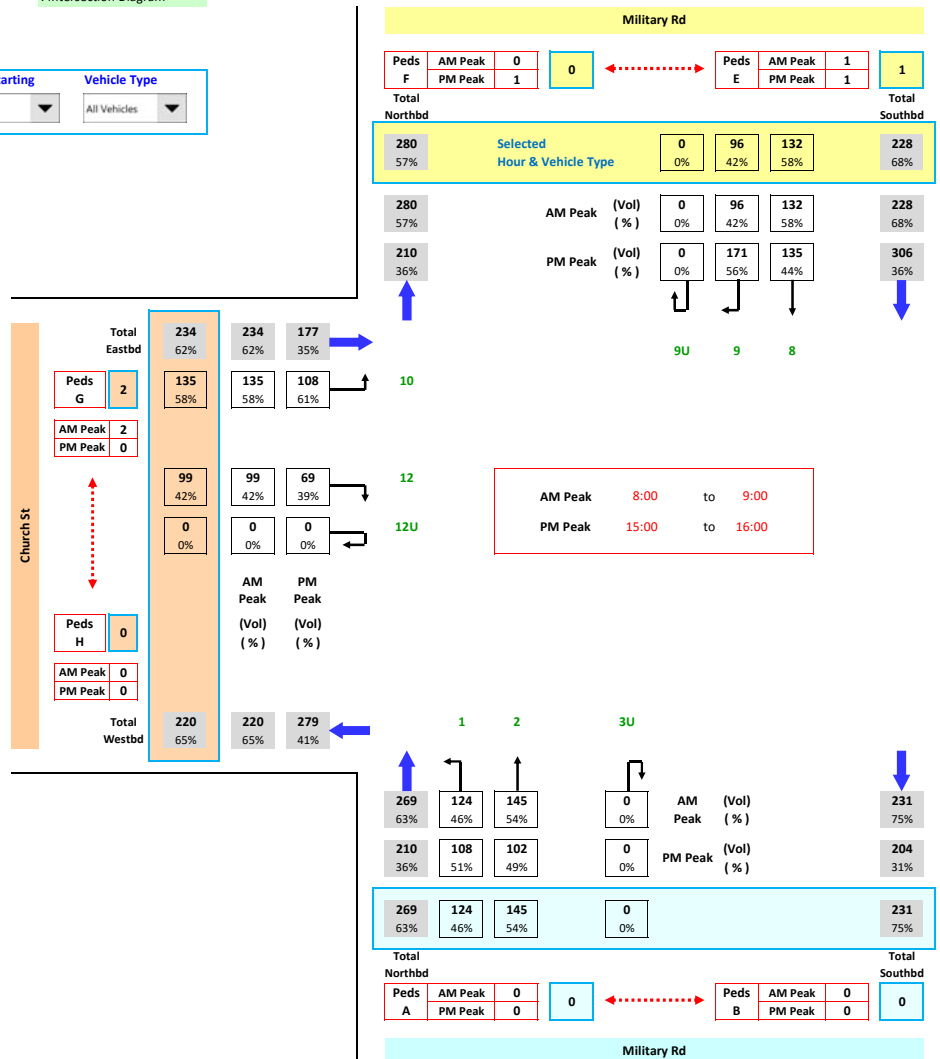


Job No. : N3855
Client : Bitzios
Suburb : Port Kembla
Location : 2. Military Rd / Church St

Day/Date : Friday, 2nd February 2018
Weather : Fine
Description : Classified Intersection Count
: Intersection Diagram



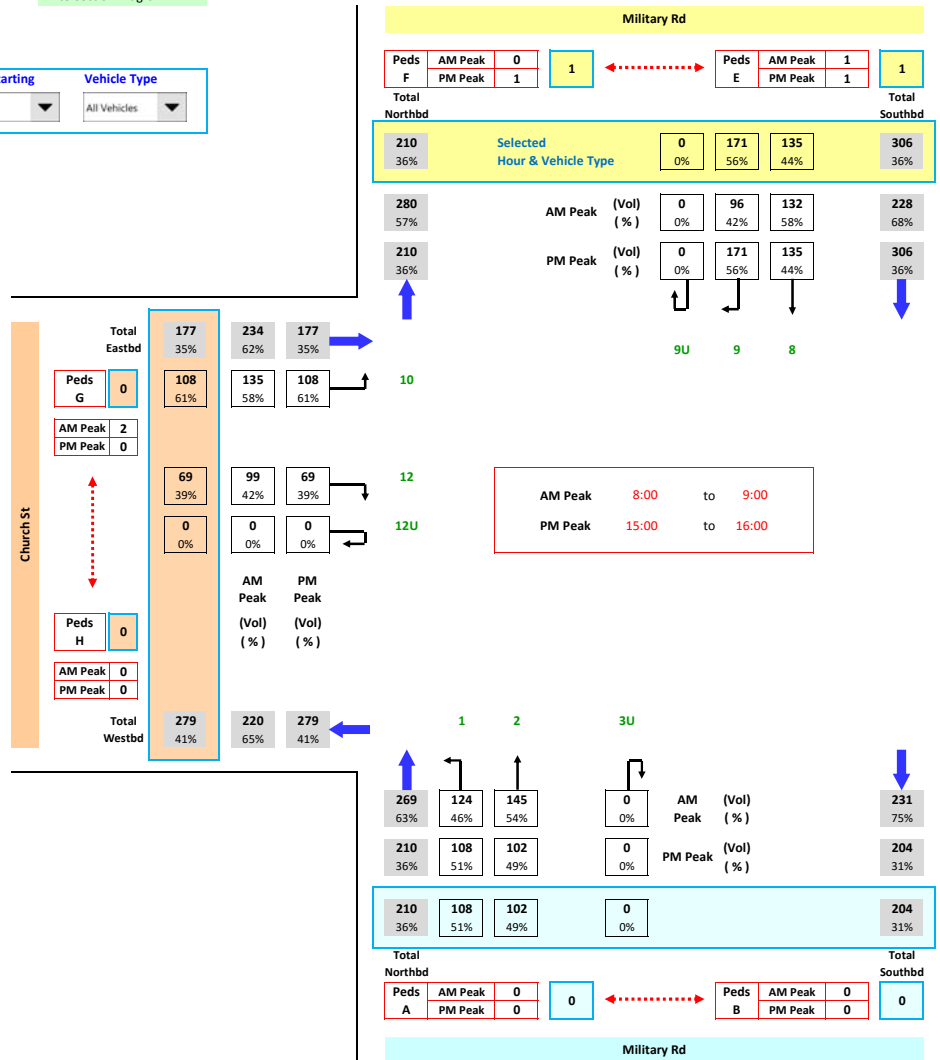
Hour Starting 8:00
Vehicle Type All Vehicles



Day/Date : Friday, 2nd February 2018
Weather : Fine
Description : Classified Intersection Count
: Intersection Diagram



Hour Starting: 15:00
Vehicle Type: All Vehicles



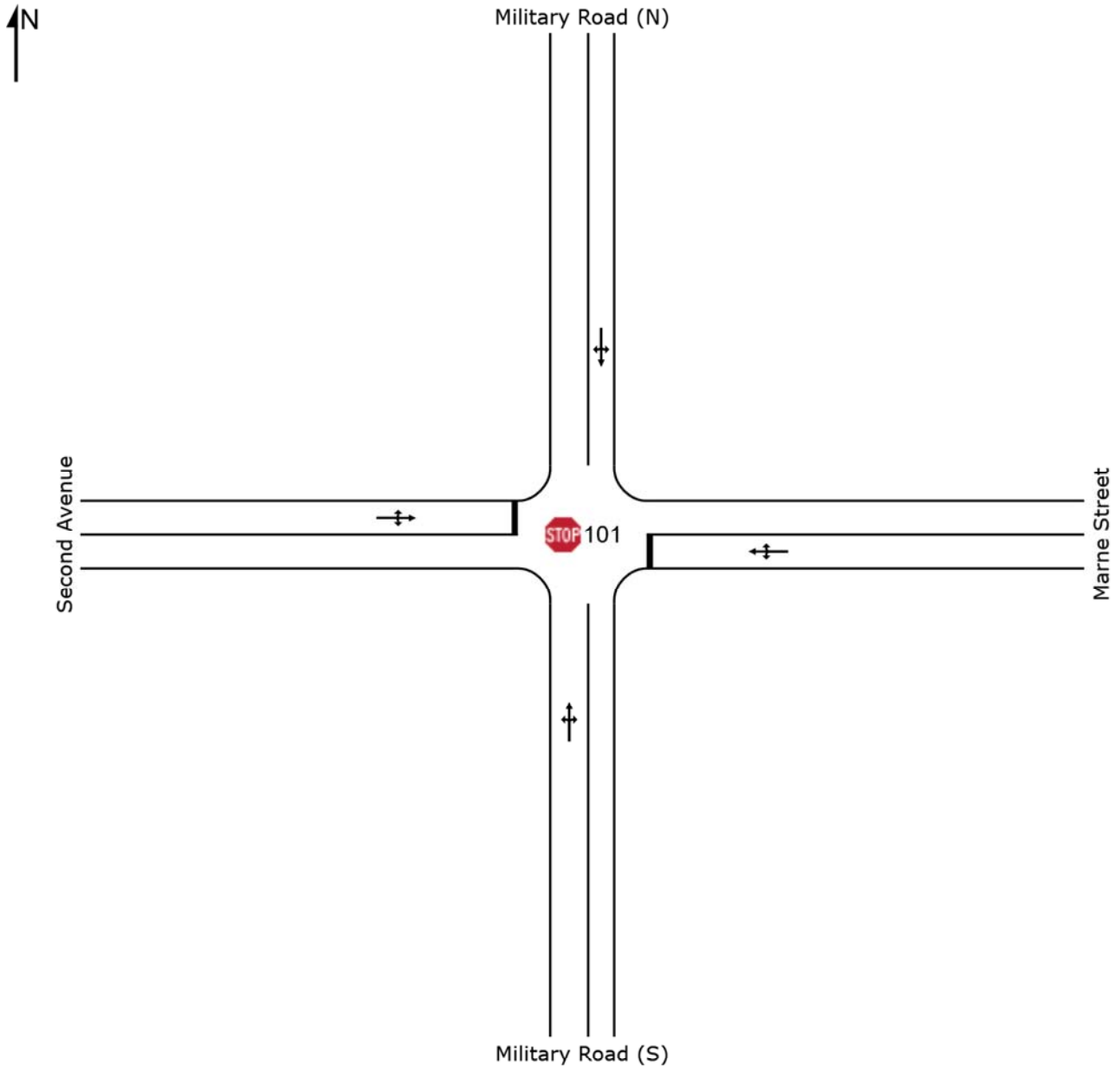
APPENDIX C

SIDRA RESULTS

SITE LAYOUT

 **Site: 101 [2019 BG AM]**

Military Road / Marne Street Unsignalised BG Intersection
Stop (Two-Way)



MOVEMENT SUMMARY

 **Site: 101 [2019 BG AM]**

Military Road / Marne Street Unsignalised BG Intersection
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------|------------|---------------|-------------------|------------------|-----------------------|---------------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 4 | 0.0 | 0.108 | 5.7 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 57.2 |
| 2 | T1 | 216 | 3.5 | 0.108 | 0.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 59.8 |
| 3 | R2 | 1 | 0.0 | 0.108 | 6.3 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 56.5 |
| Approach | | 221 | 3.4 | 0.108 | 0.1 | NA | 0.0 | 0.1 | 0.01 | 0.01 | 59.8 |
| East: Marne Street | | | | | | | | | | | |
| 4 | L2 | 1 | 0.0 | 0.019 | 8.9 | LOS A | 0.1 | 0.4 | 0.46 | 0.89 | 45.7 |
| 5 | T1 | 2 | 0.0 | 0.019 | 10.2 | LOS A | 0.1 | 0.4 | 0.46 | 0.89 | 33.0 |
| 6 | R2 | 8 | 0.0 | 0.019 | 11.3 | LOS A | 0.1 | 0.4 | 0.46 | 0.89 | 45.4 |
| Approach | | 12 | 0.0 | 0.019 | 10.9 | LOS A | 0.1 | 0.4 | 0.46 | 0.89 | 44.1 |
| North: Military Road (N) | | | | | | | | | | | |
| 7 | L2 | 8 | 0.0 | 0.134 | 6.1 | LOS A | 0.1 | 0.9 | 0.05 | 0.05 | 56.3 |
| 8 | T1 | 233 | 0.5 | 0.134 | 0.1 | LOS A | 0.1 | 0.9 | 0.05 | 0.05 | 59.3 |
| 9 | R2 | 14 | 0.0 | 0.134 | 6.3 | LOS A | 0.1 | 0.9 | 0.05 | 0.05 | 56.2 |
| Approach | | 255 | 0.5 | 0.134 | 0.6 | NA | 0.1 | 0.9 | 0.05 | 0.05 | 59.1 |
| West: Second Avenue | | | | | | | | | | | |
| 10 | L2 | 32 | 0.0 | 0.042 | 8.9 | LOS A | 0.2 | 1.1 | 0.34 | 0.87 | 47.3 |
| 11 | T1 | 3 | 0.0 | 0.042 | 10.4 | LOS A | 0.2 | 1.1 | 0.34 | 0.87 | 34.8 |
| 12 | R2 | 6 | 0.0 | 0.042 | 11.1 | LOS A | 0.2 | 1.1 | 0.34 | 0.87 | 47.0 |
| Approach | | 41 | 0.0 | 0.042 | 9.3 | LOS A | 0.2 | 1.1 | 0.34 | 0.87 | 46.7 |
| All Vehicles | | 528 | 1.6 | 0.134 | 1.3 | NA | 0.2 | 1.1 | 0.06 | 0.12 | 58.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY



Site: 101 [2019 BG PM]

Military Road / Marne Street Unsignalised BG Intersection
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------|------------|---------------|-------------------|------------------|-----------------------|---------------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 9 | 0.0 | 0.105 | 6.0 | LOS A | 0.2 | 1.2 | 0.08 | 0.08 | 55.9 |
| 2 | T1 | 178 | 5.4 | 0.105 | 0.1 | LOS A | 0.2 | 1.2 | 0.08 | 0.08 | 58.9 |
| 3 | R2 | 19 | 0.0 | 0.105 | 6.1 | LOS A | 0.2 | 1.2 | 0.08 | 0.08 | 55.1 |
| Approach | | 206 | 4.7 | 0.105 | 0.9 | NA | 0.2 | 1.2 | 0.08 | 0.08 | 58.6 |
| East: Marne Street | | | | | | | | | | | |
| 4 | L2 | 1 | 0.0 | 0.024 | 8.7 | LOS A | 0.1 | 0.6 | 0.43 | 0.89 | 46.1 |
| 5 | T1 | 1 | 0.0 | 0.024 | 9.8 | LOS A | 0.1 | 0.6 | 0.43 | 0.89 | 33.5 |
| 6 | R2 | 14 | 0.0 | 0.024 | 10.6 | LOS A | 0.1 | 0.6 | 0.43 | 0.89 | 45.8 |
| Approach | | 16 | 0.0 | 0.024 | 10.4 | LOS A | 0.1 | 0.6 | 0.43 | 0.89 | 45.3 |
| North: Military Road (N) | | | | | | | | | | | |
| 7 | L2 | 9 | 0.0 | 0.111 | 6.0 | LOS A | 0.2 | 1.1 | 0.08 | 0.08 | 55.7 |
| 8 | T1 | 178 | 3.6 | 0.111 | 0.1 | LOS A | 0.2 | 1.1 | 0.08 | 0.08 | 58.9 |
| 9 | R2 | 19 | 0.0 | 0.111 | 6.2 | LOS A | 0.2 | 1.1 | 0.08 | 0.08 | 55.6 |
| Approach | | 206 | 3.1 | 0.111 | 0.9 | NA | 0.2 | 1.1 | 0.08 | 0.08 | 58.6 |
| West: Second Avenue | | | | | | | | | | | |
| 10 | L2 | 17 | 0.0 | 0.017 | 8.7 | LOS A | 0.1 | 0.4 | 0.29 | 0.86 | 47.7 |
| 11 | T1 | 1 | 0.0 | 0.017 | 9.8 | LOS A | 0.1 | 0.4 | 0.29 | 0.86 | 35.4 |
| 12 | R2 | 1 | 0.0 | 0.017 | 10.4 | LOS A | 0.1 | 0.4 | 0.29 | 0.86 | 47.4 |
| Approach | | 19 | 0.0 | 0.017 | 8.8 | LOS A | 0.1 | 0.4 | 0.29 | 0.86 | 47.3 |
| All Vehicles | | 447 | 3.6 | 0.111 | 1.6 | NA | 0.2 | 1.2 | 0.10 | 0.14 | 57.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

 **Site: 101 [2029 BG AM]**

Military Road / Marne Street Unsignalised BG Intersection
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|-----------------------|---------------|------------------|----------------------|------------------|--------------------------------------|---------------|--------------|--------------------------------|-----------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 4 | 0.0 | 0.122 | 5.8 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 57.2 |
| 2 | T1 | 244 | 3.5 | 0.122 | 0.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 59.9 |
| 3 | R2 | 1 | 0.0 | 0.122 | 6.5 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 56.5 |
| Approach | | 249 | 3.4 | 0.122 | 0.1 | NA | 0.0 | 0.1 | 0.01 | 0.01 | 59.8 |
| East: Marne Street | | | | | | | | | | | |
| 4 | L2 | 1 | 0.0 | 0.023 | 9.0 | LOS A | 0.1 | 0.5 | 0.49 | 0.91 | 45.1 |
| 5 | T1 | 2 | 0.0 | 0.023 | 10.7 | LOS A | 0.1 | 0.5 | 0.49 | 0.91 | 32.3 |
| 6 | R2 | 9 | 0.0 | 0.023 | 12.0 | LOS A | 0.1 | 0.5 | 0.49 | 0.91 | 44.8 |
| Approach | | 13 | 0.0 | 0.023 | 11.5 | LOS A | 0.1 | 0.5 | 0.49 | 0.91 | 43.6 |
| North: Military Road (N) | | | | | | | | | | | |
| 7 | L2 | 9 | 0.0 | 0.151 | 6.2 | LOS A | 0.1 | 1.0 | 0.06 | 0.05 | 56.3 |
| 8 | T1 | 263 | 0.5 | 0.151 | 0.1 | LOS A | 0.1 | 1.0 | 0.06 | 0.05 | 59.3 |
| 9 | R2 | 15 | 0.0 | 0.151 | 6.5 | LOS A | 0.1 | 1.0 | 0.06 | 0.05 | 56.2 |
| Approach | | 287 | 0.5 | 0.151 | 0.6 | NA | 0.1 | 1.0 | 0.06 | 0.05 | 59.1 |
| West: Second Avenue | | | | | | | | | | | |
| 10 | L2 | 36 | 0.0 | 0.048 | 9.0 | LOS A | 0.2 | 1.2 | 0.37 | 0.88 | 47.1 |
| 11 | T1 | 3 | 0.0 | 0.048 | 10.9 | LOS A | 0.2 | 1.2 | 0.37 | 0.88 | 34.6 |
| 12 | R2 | 6 | 0.0 | 0.048 | 11.8 | LOS A | 0.2 | 1.2 | 0.37 | 0.88 | 46.8 |
| Approach | | 45 | 0.0 | 0.048 | 9.5 | LOS A | 0.2 | 1.2 | 0.37 | 0.88 | 46.6 |
| All Vehicles | | 595 | 1.7 | 0.151 | 1.3 | NA | 0.2 | 1.2 | 0.07 | 0.12 | 58.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

 **Site: 101 [2029 BG PM]**

Military Road / Marne Street Unsignalised BG Intersection
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------|------------|---------------|-------------------|------------------|-----------------------|---------------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 11 | 0.0 | 0.119 | 6.1 | LOS A | 0.2 | 1.4 | 0.08 | 0.08 | 55.8 |
| 2 | T1 | 201 | 5.4 | 0.119 | 0.1 | LOS A | 0.2 | 1.4 | 0.08 | 0.08 | 58.9 |
| 3 | R2 | 21 | 0.0 | 0.119 | 6.2 | LOS A | 0.2 | 1.4 | 0.08 | 0.08 | 55.1 |
| Approach | | 233 | 4.7 | 0.119 | 0.9 | NA | 0.2 | 1.4 | 0.08 | 0.08 | 58.5 |
| East: Marne Street | | | | | | | | | | | |
| 4 | L2 | 1 | 0.0 | 0.028 | 8.8 | LOS A | 0.1 | 0.6 | 0.46 | 0.90 | 45.6 |
| 5 | T1 | 1 | 0.0 | 0.028 | 10.2 | LOS A | 0.1 | 0.6 | 0.46 | 0.90 | 33.0 |
| 6 | R2 | 15 | 0.0 | 0.028 | 11.1 | LOS A | 0.1 | 0.6 | 0.46 | 0.90 | 45.4 |
| Approach | | 17 | 0.0 | 0.028 | 10.9 | LOS A | 0.1 | 0.6 | 0.46 | 0.90 | 44.9 |
| North: Military Road (N) | | | | | | | | | | | |
| 7 | L2 | 11 | 0.0 | 0.126 | 6.1 | LOS A | 0.2 | 1.3 | 0.08 | 0.08 | 55.7 |
| 8 | T1 | 201 | 3.6 | 0.126 | 0.1 | LOS A | 0.2 | 1.3 | 0.08 | 0.08 | 58.9 |
| 9 | R2 | 21 | 0.0 | 0.126 | 6.3 | LOS A | 0.2 | 1.3 | 0.08 | 0.08 | 55.6 |
| Approach | | 233 | 3.1 | 0.126 | 0.9 | NA | 0.2 | 1.3 | 0.08 | 0.08 | 58.6 |
| West: Second Avenue | | | | | | | | | | | |
| 10 | L2 | 19 | 0.0 | 0.019 | 8.8 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 47.6 |
| 11 | T1 | 1 | 0.0 | 0.019 | 10.2 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 35.3 |
| 12 | R2 | 1 | 0.0 | 0.019 | 10.9 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 47.3 |
| Approach | | 21 | 0.0 | 0.019 | 9.0 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 47.2 |
| All Vehicles | | 503 | 3.6 | 0.126 | 1.6 | NA | 0.2 | 1.4 | 0.11 | 0.14 | 57.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

 **Site: 101 [2029 DES AM]**

Military Road / Marne Street Unsignalised BG Intersection
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------|------------|---------------|-------------------|------------------|-----------------------|---------------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 4 | 0.0 | 0.122 | 5.8 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 57.2 |
| 2 | T1 | 244 | 3.5 | 0.122 | 0.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 59.9 |
| 3 | R2 | 1 | 0.0 | 0.122 | 6.5 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 56.5 |
| Approach | | 249 | 3.4 | 0.122 | 0.1 | NA | 0.0 | 0.1 | 0.01 | 0.01 | 59.8 |
| East: Marne Street | | | | | | | | | | | |
| 4 | L2 | 3 | 0.0 | 0.130 | 9.2 | LOS A | 0.4 | 3.1 | 0.53 | 0.99 | 44.4 |
| 5 | T1 | 2 | 0.0 | 0.130 | 11.2 | LOS A | 0.4 | 3.1 | 0.53 | 0.99 | 31.4 |
| 6 | R2 | 63 | 0.0 | 0.130 | 12.5 | LOS A | 0.4 | 3.1 | 0.53 | 0.99 | 44.2 |
| Approach | | 68 | 0.0 | 0.130 | 12.3 | LOS A | 0.4 | 3.1 | 0.53 | 0.99 | 44.0 |
| North: Military Road (N) | | | | | | | | | | | |
| 7 | L2 | 24 | 0.0 | 0.159 | 5.9 | LOS A | 0.2 | 1.1 | 0.06 | 0.07 | 56.0 |
| 8 | T1 | 263 | 0.5 | 0.159 | 0.1 | LOS A | 0.2 | 1.1 | 0.06 | 0.07 | 59.1 |
| 9 | R2 | 15 | 0.0 | 0.159 | 6.5 | LOS A | 0.2 | 1.1 | 0.06 | 0.07 | 55.9 |
| Approach | | 302 | 0.4 | 0.159 | 0.9 | NA | 0.2 | 1.1 | 0.06 | 0.07 | 58.8 |
| West: Second Avenue | | | | | | | | | | | |
| 10 | L2 | 36 | 0.0 | 0.049 | 9.0 | LOS A | 0.2 | 1.3 | 0.37 | 0.88 | 47.1 |
| 11 | T1 | 3 | 0.0 | 0.049 | 11.1 | LOS A | 0.2 | 1.3 | 0.37 | 0.88 | 34.6 |
| 12 | R2 | 6 | 0.0 | 0.049 | 11.8 | LOS A | 0.2 | 1.3 | 0.37 | 0.88 | 46.8 |
| Approach | | 45 | 0.0 | 0.049 | 9.6 | LOS A | 0.2 | 1.3 | 0.37 | 0.88 | 46.6 |
| All Vehicles | | 665 | 1.5 | 0.159 | 2.4 | NA | 0.4 | 3.1 | 0.11 | 0.20 | 57.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

 **Site: 101 [2029 DES PM]**

Military Road / Marne Street Unsignalised BG Intersection
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------|------------|---------------|-------------------|------------------|--------------------------------|------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 11 | 0.0 | 0.122 | 6.3 | LOS A | 0.2 | 1.6 | 0.11 | 0.09 | 55.6 |
| 2 | T1 | 201 | 5.4 | 0.122 | 0.2 | LOS A | 0.2 | 1.6 | 0.11 | 0.09 | 58.7 |
| 3 | R2 | 24 | 0.0 | 0.122 | 6.4 | LOS A | 0.2 | 1.6 | 0.11 | 0.09 | 54.9 |
| Approach | | 236 | 4.6 | 0.122 | 1.1 | NA | 0.2 | 1.6 | 0.11 | 0.09 | 58.4 |
| East: Marne Street | | | | | | | | | | | |
| 4 | L2 | 2 | 0.0 | 0.067 | 8.8 | LOS A | 0.2 | 1.6 | 0.48 | 0.94 | 45.3 |
| 5 | T1 | 1 | 0.0 | 0.067 | 10.6 | LOS A | 0.2 | 1.6 | 0.48 | 0.94 | 32.5 |
| 6 | R2 | 36 | 0.0 | 0.067 | 11.5 | LOS A | 0.2 | 1.6 | 0.48 | 0.94 | 45.0 |
| Approach | | 39 | 0.0 | 0.067 | 11.3 | LOS A | 0.2 | 1.6 | 0.48 | 0.94 | 44.8 |
| North: Military Road (N) | | | | | | | | | | | |
| 7 | L2 | 58 | 0.0 | 0.151 | 5.8 | LOS A | 0.2 | 1.5 | 0.08 | 0.16 | 54.6 |
| 8 | T1 | 201 | 3.6 | 0.151 | 0.1 | LOS A | 0.2 | 1.5 | 0.08 | 0.16 | 58.1 |
| 9 | R2 | 21 | 0.0 | 0.151 | 6.3 | LOS A | 0.2 | 1.5 | 0.08 | 0.16 | 54.6 |
| Approach | | 280 | 2.6 | 0.151 | 1.7 | NA | 0.2 | 1.5 | 0.08 | 0.16 | 57.4 |
| West: Second Avenue | | | | | | | | | | | |
| 10 | L2 | 19 | 0.0 | 0.019 | 8.8 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 47.6 |
| 11 | T1 | 1 | 0.0 | 0.019 | 10.6 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 35.3 |
| 12 | R2 | 1 | 0.0 | 0.019 | 10.9 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 47.3 |
| Approach | | 21 | 0.0 | 0.019 | 9.0 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 47.2 |
| All Vehicles | | 576 | 3.1 | 0.151 | 2.4 | NA | 0.2 | 1.6 | 0.13 | 0.21 | 56.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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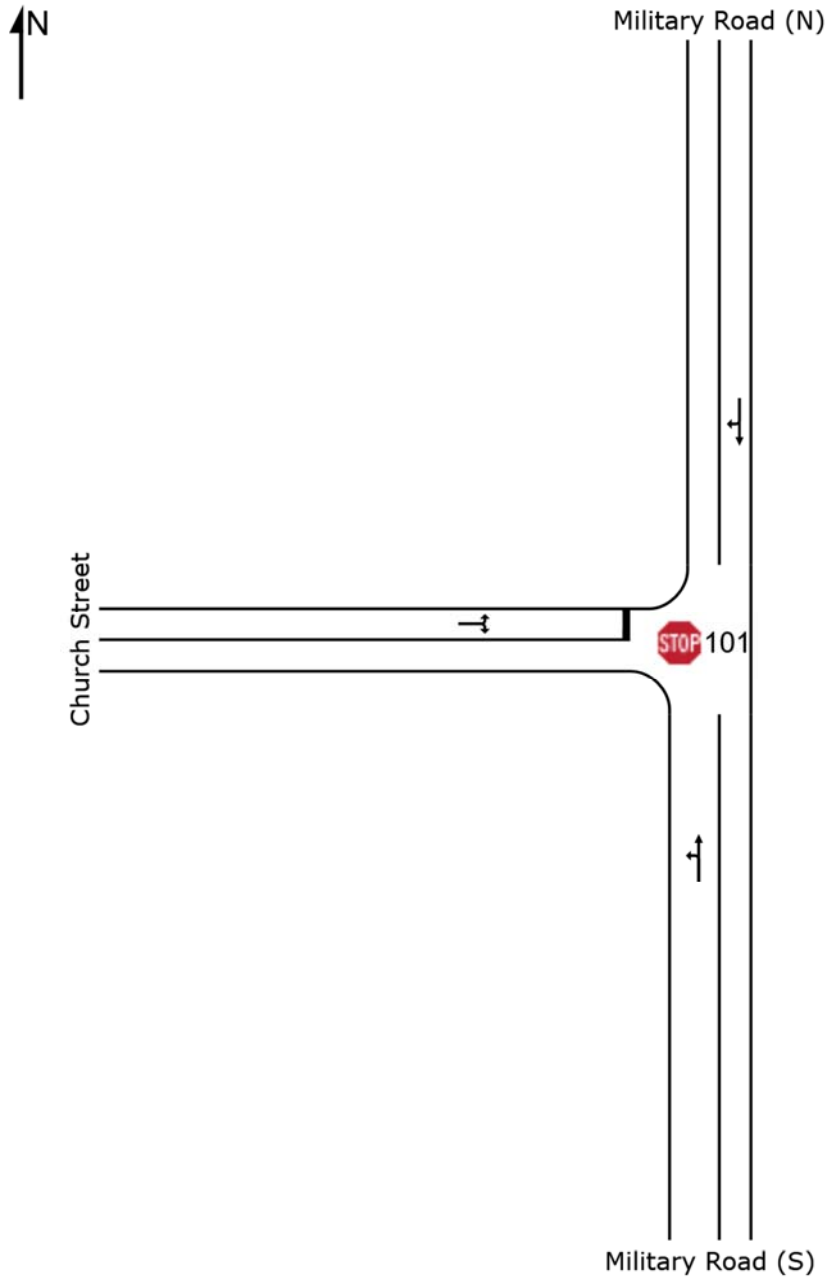
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SITE LAYOUT

 **Site: 101 [2019 BG AM]**

Military Road Church Street Unsignalised
Stop (Two-Way)



MOVEMENT SUMMARY



Site: 101 [2019 BG AM]

Military Road Church Street Unsignalised
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------------|------------|---------------|-------------------|------------------|--------------------------------|------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Flows Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 133 | 4.0 | 0.142 | 4.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.26 | 31.2 |
| 2 | T1 | 156 | 1.4 | 0.142 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.26 | 49.4 |
| Approach | | 288 | 2.6 | 0.142 | 2.0 | NA | 0.0 | 0.0 | 0.00 | 0.26 | 38.4 |
| North: Military Road (N) | | | | | | | | | | | |
| 8 | T1 | 141 | 0.8 | 0.155 | 0.8 | LOS A | 0.7 | 5.3 | 0.34 | 0.26 | 43.8 |
| 9 | R2 | 103 | 8.3 | 0.155 | 6.3 | LOS A | 0.7 | 5.3 | 0.34 | 0.26 | 31.6 |
| Approach | | 244 | 4.0 | 0.155 | 3.1 | NA | 0.7 | 5.3 | 0.34 | 0.26 | 36.9 |
| West: Church Street | | | | | | | | | | | |
| 10 | L2 | 145 | 5.9 | 0.276 | 9.0 | LOS A | 1.2 | 8.4 | 0.35 | 0.92 | 29.8 |
| 12 | R2 | 106 | 4.0 | 0.276 | 11.1 | LOS A | 1.2 | 8.4 | 0.35 | 0.92 | 28.1 |
| Approach | | 252 | 5.1 | 0.276 | 9.9 | LOS A | 1.2 | 8.4 | 0.35 | 0.92 | 29.1 |
| All Vehicles | | 784 | 3.8 | 0.276 | 4.9 | NA | 1.2 | 8.4 | 0.22 | 0.48 | 34.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY



Site: 101 [2019 BG PM]

Military Road Church Street Unsignalised
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|-----------------------|---------------|------------------|----------------------|------------------|--------------------------------------|---------------|--------------|--------------------------------|-----------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 116 | 4.0 | 0.111 | 4.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.30 | 30.9 |
| 2 | T1 | 109 | 1.4 | 0.111 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.30 | 48.4 |
| Approach | | 225 | 2.7 | 0.111 | 2.2 | NA | 0.0 | 0.0 | 0.00 | 0.30 | 37.0 |
| North: Military Road (N) | | | | | | | | | | | |
| 8 | T1 | 145 | 0.8 | 0.211 | 0.8 | LOS A | 1.1 | 8.1 | 0.35 | 0.34 | 41.4 |
| 9 | R2 | 183 | 8.3 | 0.211 | 6.0 | LOS A | 1.1 | 8.1 | 0.35 | 0.34 | 30.6 |
| Approach | | 328 | 5.0 | 0.211 | 3.7 | NA | 1.1 | 8.1 | 0.35 | 0.34 | 34.0 |
| West: Church Street | | | | | | | | | | | |
| 10 | L2 | 116 | 5.9 | 0.206 | 8.7 | LOS A | 0.8 | 6.0 | 0.27 | 0.93 | 29.8 |
| 12 | R2 | 75 | 4.0 | 0.206 | 11.3 | LOS A | 0.8 | 6.0 | 0.27 | 0.93 | 28.2 |
| Approach | | 191 | 5.2 | 0.206 | 9.7 | LOS A | 0.8 | 6.0 | 0.27 | 0.93 | 29.2 |
| All Vehicles | | 744 | 4.3 | 0.211 | 4.8 | NA | 1.1 | 8.1 | 0.22 | 0.48 | 33.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY



Site: 101 [2029 BG AM]

Military Road Church Street Unsignalised
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------------|------------|---------------|-------------------|------------------|--------------------------------|------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Flows Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 151 | 4.0 | 0.160 | 4.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.27 | 31.2 |
| 2 | T1 | 176 | 1.4 | 0.160 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.27 | 49.4 |
| Approach | | 326 | 2.6 | 0.160 | 2.0 | NA | 0.0 | 0.0 | 0.00 | 0.27 | 38.4 |
| North: Military Road (N) | | | | | | | | | | | |
| 8 | T1 | 160 | 0.8 | 0.180 | 1.0 | LOS A | 0.9 | 6.3 | 0.37 | 0.27 | 43.3 |
| 9 | R2 | 117 | 8.3 | 0.180 | 6.5 | LOS A | 0.9 | 6.3 | 0.37 | 0.27 | 31.4 |
| Approach | | 277 | 4.0 | 0.180 | 3.3 | NA | 0.9 | 6.3 | 0.37 | 0.27 | 36.7 |
| West: Church Street | | | | | | | | | | | |
| 10 | L2 | 164 | 5.9 | 0.328 | 9.3 | LOS A | 1.5 | 10.9 | 0.39 | 0.93 | 29.2 |
| 12 | R2 | 120 | 4.0 | 0.328 | 12.0 | LOS A | 1.5 | 10.9 | 0.39 | 0.93 | 27.6 |
| Approach | | 284 | 5.1 | 0.328 | 10.4 | LOS A | 1.5 | 10.9 | 0.39 | 0.93 | 28.5 |
| All Vehicles | | 887 | 3.8 | 0.328 | 5.1 | NA | 1.5 | 10.9 | 0.24 | 0.48 | 33.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY



Site: 101 [2029 BG PM]

Military Road Church Street Unsignalised
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|-----------------------|---------------|------------------|----------------------|------------------|--------------------------------------|---------------|--------------|--------------------------------|-----------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 132 | 4.0 | 0.126 | 4.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.30 | 30.9 |
| 2 | T1 | 124 | 1.4 | 0.126 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.30 | 48.4 |
| Approach | | 256 | 2.7 | 0.126 | 2.2 | NA | 0.0 | 0.0 | 0.00 | 0.30 | 37.0 |
| North: Military Road (N) | | | | | | | | | | | |
| 8 | T1 | 164 | 0.8 | 0.244 | 1.0 | LOS A | 1.3 | 9.7 | 0.39 | 0.35 | 41.0 |
| 9 | R2 | 207 | 8.3 | 0.244 | 6.2 | LOS A | 1.3 | 9.7 | 0.39 | 0.35 | 30.4 |
| Approach | | 372 | 5.0 | 0.244 | 3.9 | NA | 1.3 | 9.7 | 0.39 | 0.35 | 33.8 |
| West: Church Street | | | | | | | | | | | |
| 10 | L2 | 132 | 5.9 | 0.247 | 8.8 | LOS A | 1.0 | 7.3 | 0.30 | 0.92 | 29.4 |
| 12 | R2 | 84 | 4.0 | 0.247 | 12.1 | LOS A | 1.0 | 7.3 | 0.30 | 0.92 | 27.8 |
| Approach | | 216 | 5.2 | 0.247 | 10.1 | LOS A | 1.0 | 7.3 | 0.30 | 0.92 | 28.8 |
| All Vehicles | | 843 | 4.3 | 0.247 | 5.0 | NA | 1.3 | 9.7 | 0.25 | 0.48 | 33.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY



Site: 101 [2029 DES AM]

Military Road Church Street Unsignalised
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|--------------------------|------------|---------------|-------------------|------------------|--------------------------------|------------|--------------|-----------------------------|--------------------|
| Mov ID | OD Mov | Demand Flows Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 176 | 4.0 | 0.187 | 4.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.27 | 31.2 |
| 2 | T1 | 204 | 1.4 | 0.187 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.27 | 49.3 |
| Approach | | 380 | 2.6 | 0.187 | 2.0 | NA | 0.0 | 0.0 | 0.00 | 0.27 | 38.4 |
| North: Military Road (N) | | | | | | | | | | | |
| 8 | T1 | 167 | 0.8 | 0.190 | 1.2 | LOS A | 0.9 | 6.8 | 0.40 | 0.27 | 43.0 |
| 9 | R2 | 117 | 8.3 | 0.190 | 6.9 | LOS A | 0.9 | 6.8 | 0.40 | 0.27 | 31.3 |
| Approach | | 284 | 3.9 | 0.190 | 3.5 | NA | 0.9 | 6.8 | 0.40 | 0.27 | 36.5 |
| West: Church Street | | | | | | | | | | | |
| 10 | L2 | 164 | 5.9 | 0.356 | 9.7 | LOS A | 1.8 | 12.8 | 0.44 | 0.95 | 28.6 |
| 12 | R2 | 127 | 4.0 | 0.356 | 12.9 | LOS A | 1.8 | 12.8 | 0.44 | 0.95 | 26.9 |
| Approach | | 292 | 5.1 | 0.356 | 11.1 | LOS A | 1.8 | 12.8 | 0.44 | 0.95 | 27.9 |
| All Vehicles | | 956 | 3.7 | 0.356 | 5.2 | NA | 1.8 | 12.8 | 0.25 | 0.48 | 33.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY



Site: 101 [2029 DES PM]

Military Road Church Street Unsignalised
Stop (Two-Way)

| Movement Performance - Vehicles | | | | | | | | | | | |
|---------------------------------|--------|-----------------------|---------------|------------------|----------------------|------------------|--------------------------------------|---------------|--------------|--------------------------------|-----------------------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back of Queue Vehicles veh | Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Military Road (S) | | | | | | | | | | | |
| 1 | L2 | 142 | 4.0 | 0.137 | 4.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.29 | 30.9 |
| 2 | T1 | 136 | 1.4 | 0.137 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.29 | 48.4 |
| Approach | | 278 | 2.7 | 0.137 | 2.2 | NA | 0.0 | 0.0 | 0.00 | 0.29 | 37.1 |
| North: Military Road (N) | | | | | | | | | | | |
| 8 | T1 | 189 | 0.8 | 0.261 | 1.0 | LOS A | 1.4 | 10.4 | 0.40 | 0.33 | 41.4 |
| 9 | R2 | 207 | 8.3 | 0.261 | 6.4 | LOS A | 1.4 | 10.4 | 0.40 | 0.33 | 30.6 |
| Approach | | 397 | 4.7 | 0.261 | 3.8 | NA | 1.4 | 10.4 | 0.40 | 0.33 | 34.3 |
| West: Church Street | | | | | | | | | | | |
| 10 | L2 | 132 | 5.9 | 0.297 | 8.9 | LOS A | 1.2 | 9.0 | 0.34 | 0.93 | 28.8 |
| 12 | R2 | 107 | 4.0 | 0.297 | 12.8 | LOS A | 1.2 | 9.0 | 0.34 | 0.93 | 27.1 |
| Approach | | 239 | 5.0 | 0.297 | 10.7 | LOS A | 1.2 | 9.0 | 0.34 | 0.93 | 28.1 |
| All Vehicles | | 914 | 4.2 | 0.297 | 5.1 | NA | 1.4 | 10.4 | 0.26 | 0.48 | 33.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

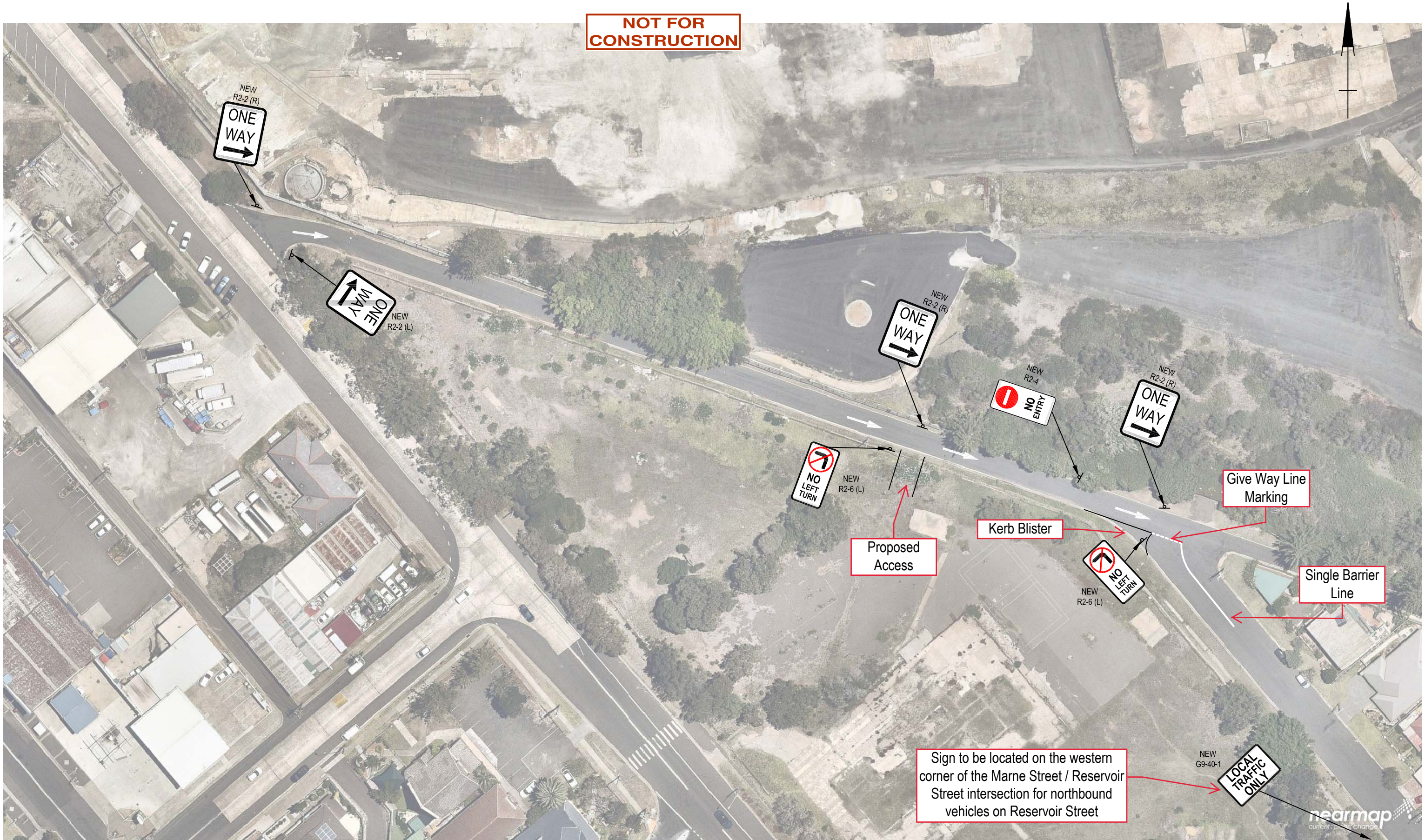
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Project: P:\P3452 Military Road Port Kembla Planning Proposal TIA\Technical Work\Models\P3452 Military Road - Church Street Intersection.sip7

APPENDIX D

ELECTROLYTIC STREET MODIFICATION



| | | | |
|----------------------|---|--|----------------------------|
| Date: 2/03/18 | Drawing Name: Electrolytic Street Modification | | |
| Project No: P3452 | Project Name: Military Road Port Kembla TIA | | Sheet 1 Version A |